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APPENDIX 'A' GEOTECHNICAL REPORT

City of Winnipeg McGillivray Boulevard Widening Geotechnical Investigation

Prepared by: UMA Engineering Ltd. 1479 Buffalo Place Winnipeg, Manitoba R3T 1L7

UMA Project No: 0265 406 00 21 00 (4.4.2)

May, 2008

UMA Engineering Ltd.

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May 23, 2008

UMA Project No.: 0265 406 00 (4.4.2)

Mr. Ron Bruce, P.Eng UMA Engineering Ltd. 2 – 1600 Ness Avenue Madison Square Winnipeg, Manitoba R3J 3W7

Dear Sir:

Re: McGillivray Boulevard Widening Field and Laboratory Investigation

UMA Engineering Ltd. (UMA) is pleased to present our report on the above referenced project. If you have any questions please do not hesitate to contact Nelson Ferreira or Ryan Belbas of our office.

Yours truly,

UMA Engineering Ltd.

Ron Typliski, P.Eng. Regional Manager

Earth and Environmental

/dh

CITY OF WINNIPEG McGILLIVRAY BOULEVARD WIDENING GEOTECHNICAL INVESTIGATION



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1.0 Summary

This report summarizes the results of the subsurface investigation completed for the proposed McGillivray Boulevard Widening project between Waverley Street and Dunkley Street. The project consists of reconstruction of the existing road and construction of a new north road embankment. Information regarding the concrete, asphalt, road base for the existing road embankment and the soil stratigraphy underneath the existing road and along the proposed north road embankment is provided.

2.0 Field Investigation and Laboratory Program

A total of 29 test holes were drilled. Nine of the test holes were drilled through the shoulder of the existing road embankment and twenty test holes were drilled along the proposed north embankment alignment. The original field program consisted of 22 test holes (TH-8-01 to TH08-22) with several optional test holes to be drilled if differing soil conditions were encountered. A total of 7 optional test holes (TH08-A, TH08-C to TH08-F, TH08-H and TH08-I) were drilled due to varying soil conditions, in particular at locations where a silt layer was present near ground surface. Since the shoulder test holes could not be drilled through the existing pavement due to traffic constraints, 8 pavement holes (PH08-01, PH08-03, PH08-05, PH08-10, PH08-15, PH08-17, PH08-19, PH08-21) were drilled near adjacent shoulder test holes to determine the condition and thickness of the road structure. The test hole and pavement hole locations are illustrated on Figures 01 to 08 and the associated logs are included in Appendix A.

The test holes were drilled to a depth between 2.3 m and 3.5 m by Paddock Drilling Ltd. using either a truck mounted CT-250 drill rig or a track mounted Yanmar C25R equipped with 150 mm diameter solid stem augers. The soils and subsurface conditions observed during drilling of the test holes were visually classified by Ryan Belbas, Geotechnical EIT of UMA. Other pertinent information such as groundwater and drilling conditions were recorded during drilling. Disturbed (auger cuttings) samples retrieved during the field investigation were transported to UMA's material testing laboratory for further testing.

The pavement holes were cored using a diamond core drill equipped with a 110 mm diameter core bit. The road base was drilled through the cored pavement using a Hilti hammer drill equipped with a 0.6 m long, 50mm diameter drill bit. Matt Lotecki of UMA supervised the coring of the road structure and drilling of the road base. Core samples and drill cuttings retrieved during the field investigation were transported to UMA's material testing laboratory for classification. The thickness of the road base could not determined at 5 of the 8 pavement hole locations as the base of the layer was beyond the reach of the drill bit used. In this regard, the maximum depth of the road base drilled is represented by a dashed line on the pavement hole logs at these locations. The photos of each core sample are included in Appendix B.

The laboratory testing program consisted of moisture content determination, Atterberg limits and hydrometer test. The laboratory information has been included on the test hole logs and a summary table of the laboratory testing results has been included in Appendix A.

Respectfully submitted,

UMA Engineering Ltd.

Reviewed by:

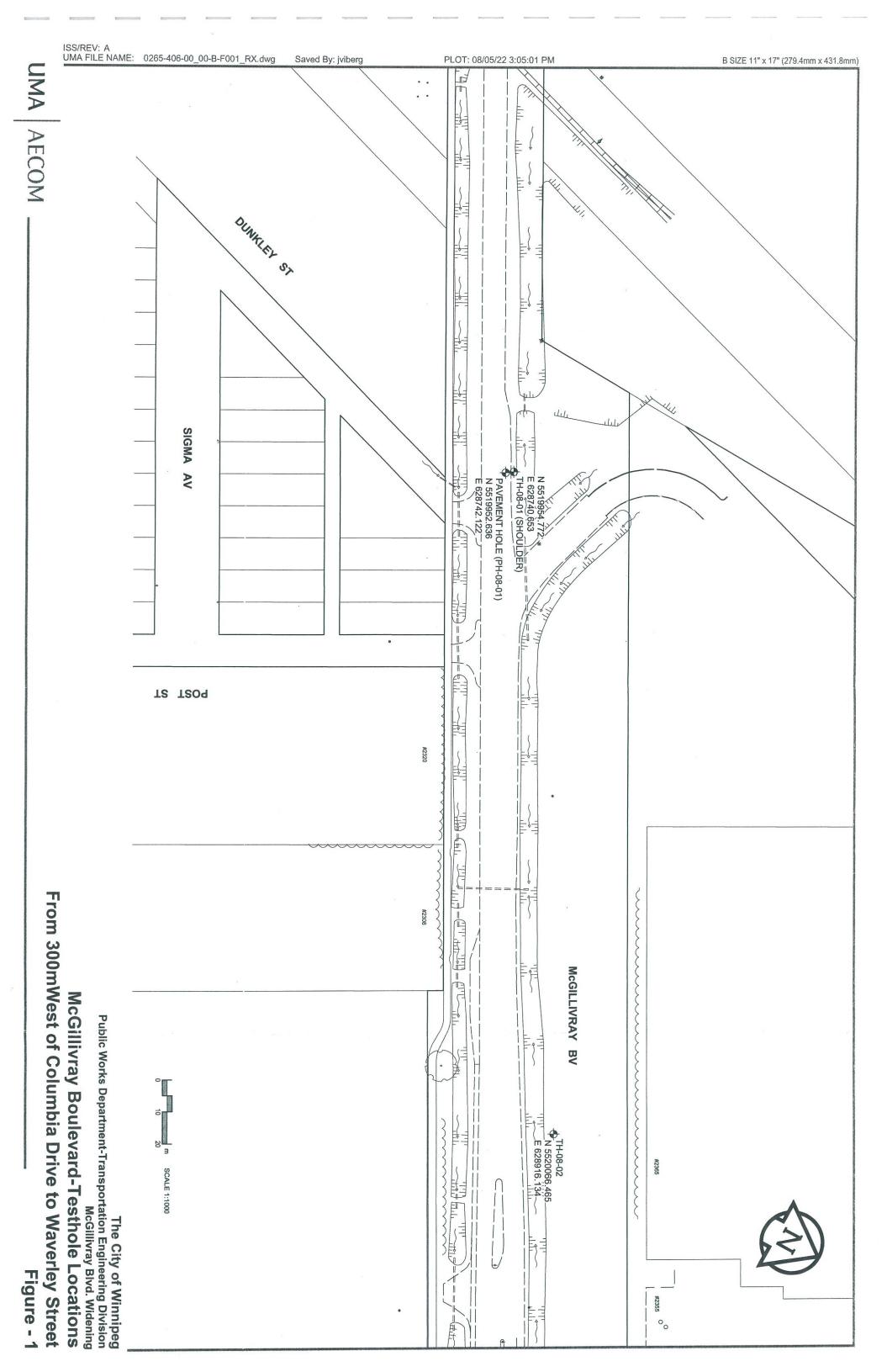
Ryan Belbas, B.Sc., EIT Geotechnical Engineering Earth and Environmental

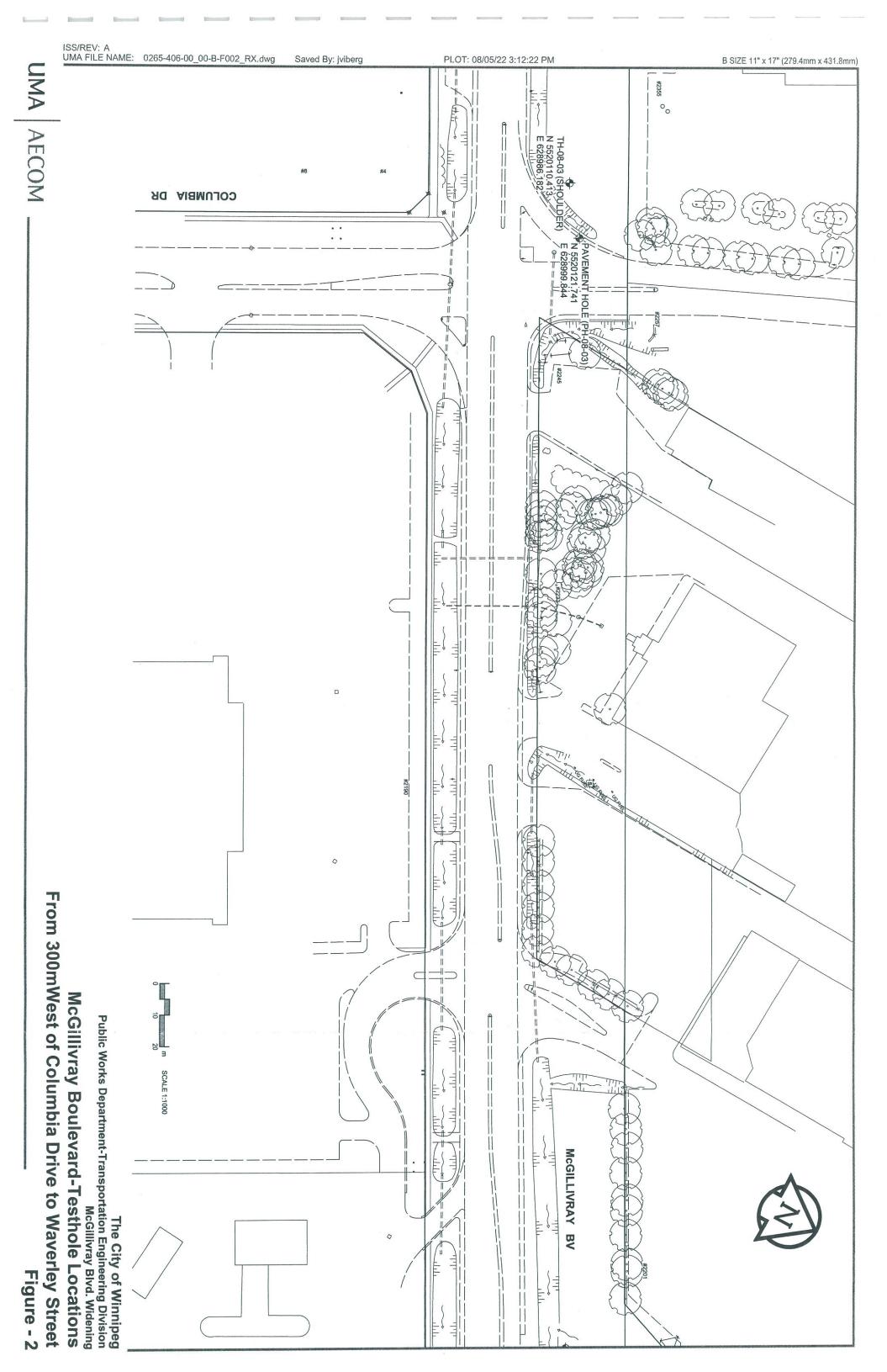
Geotechnical Engineer Earth and Environmental

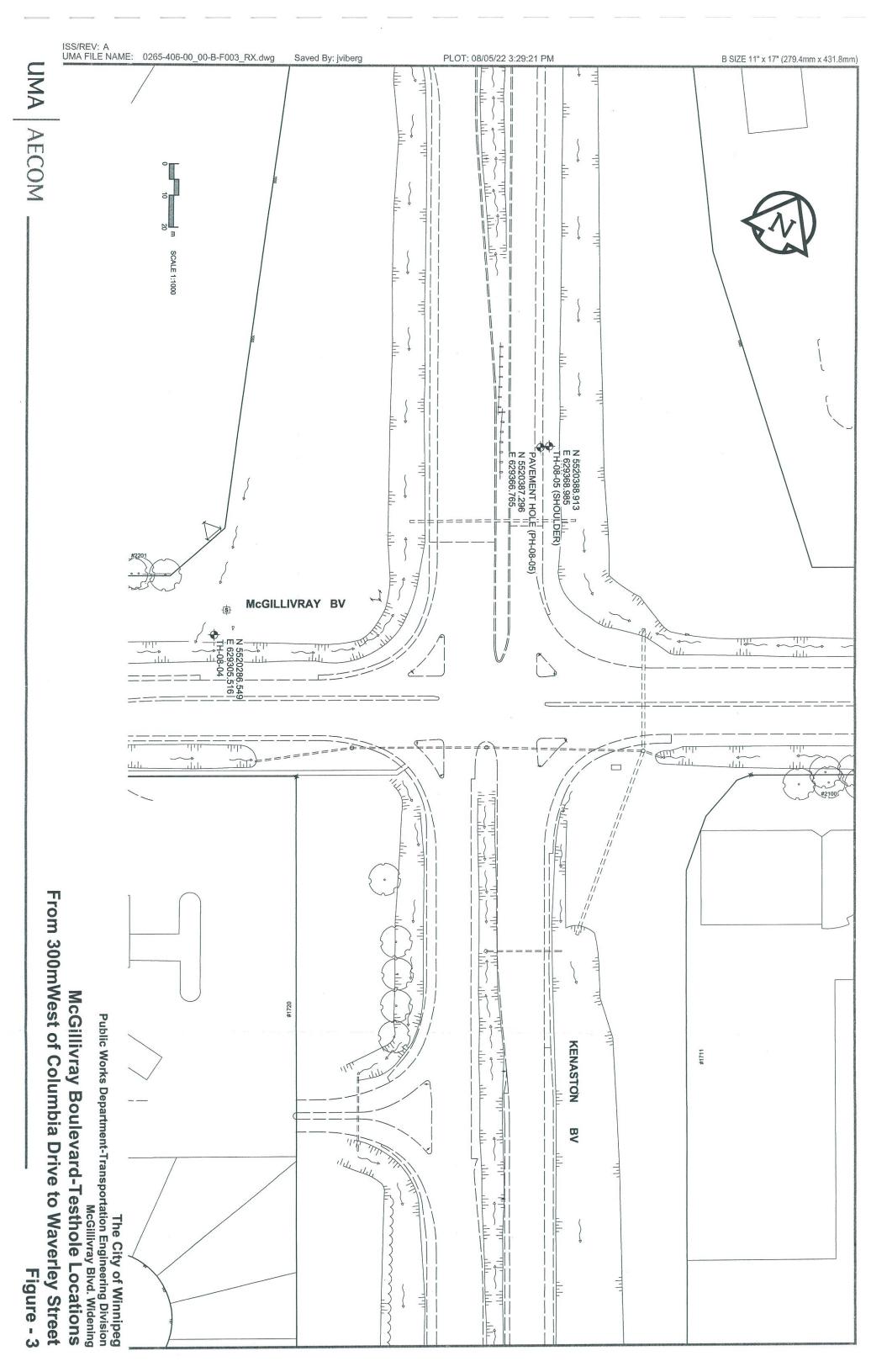
Nelson Ferreira, M.Sc., P.Eng.

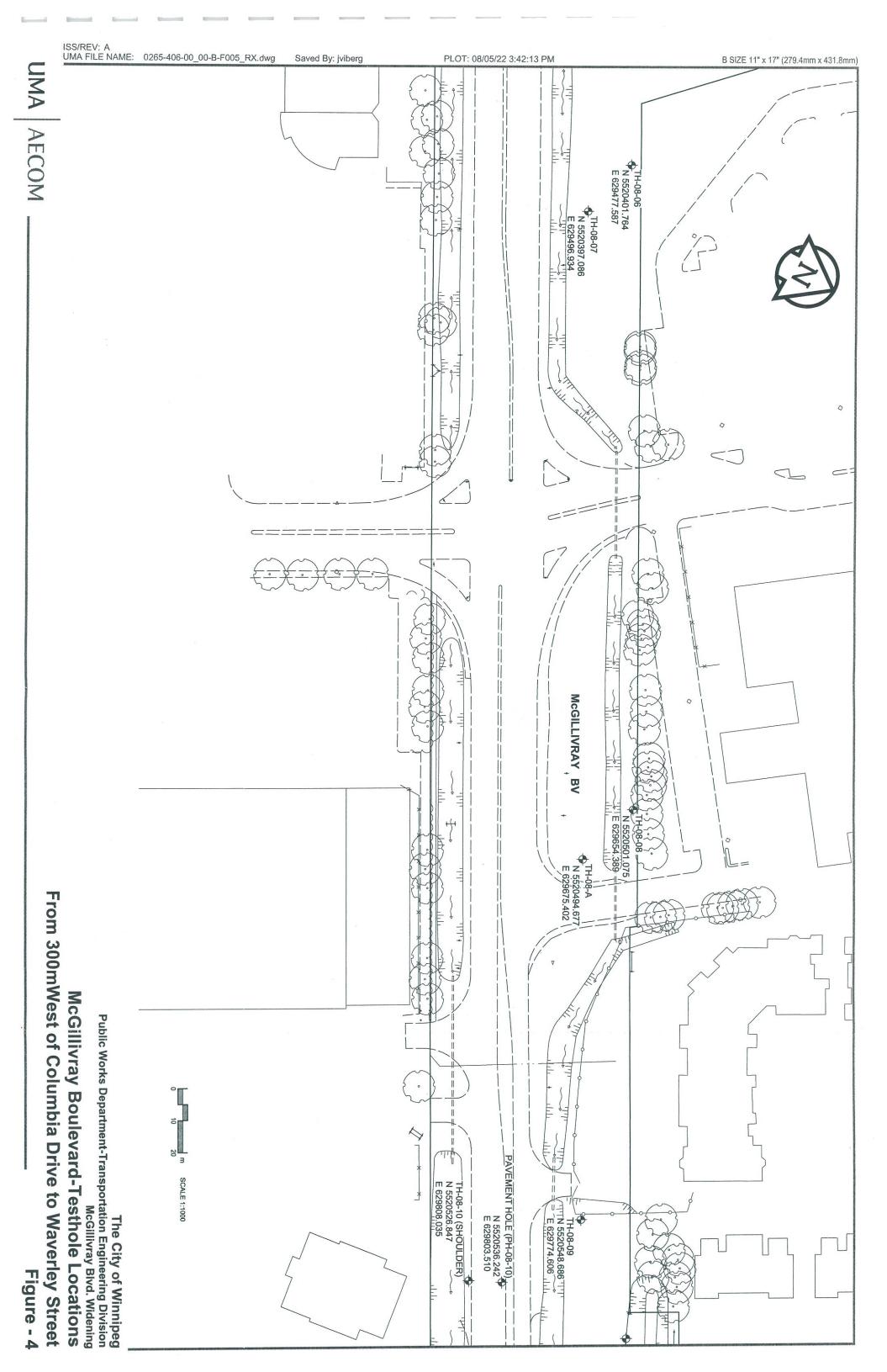
Figures

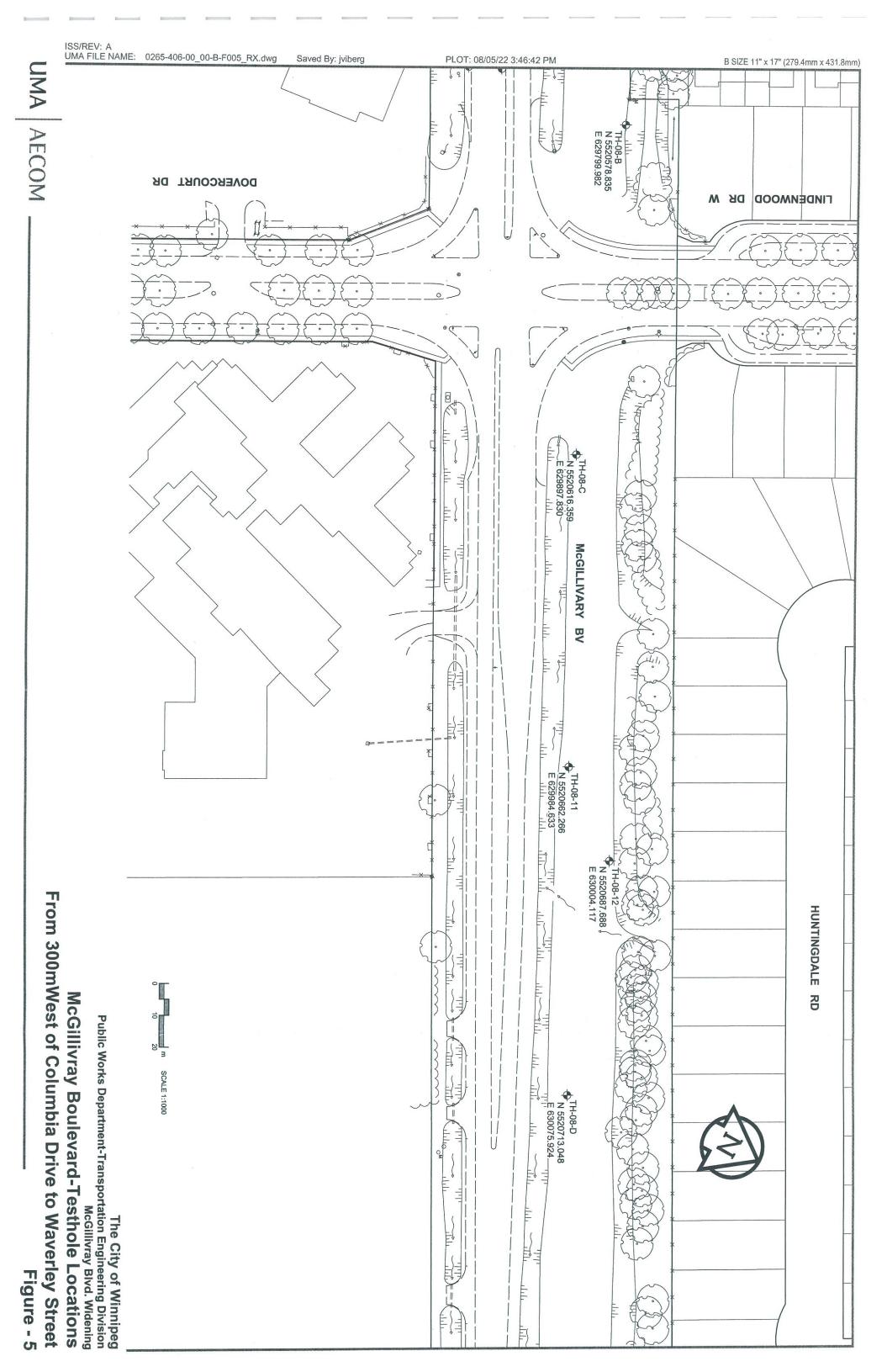
Test Hole Plan

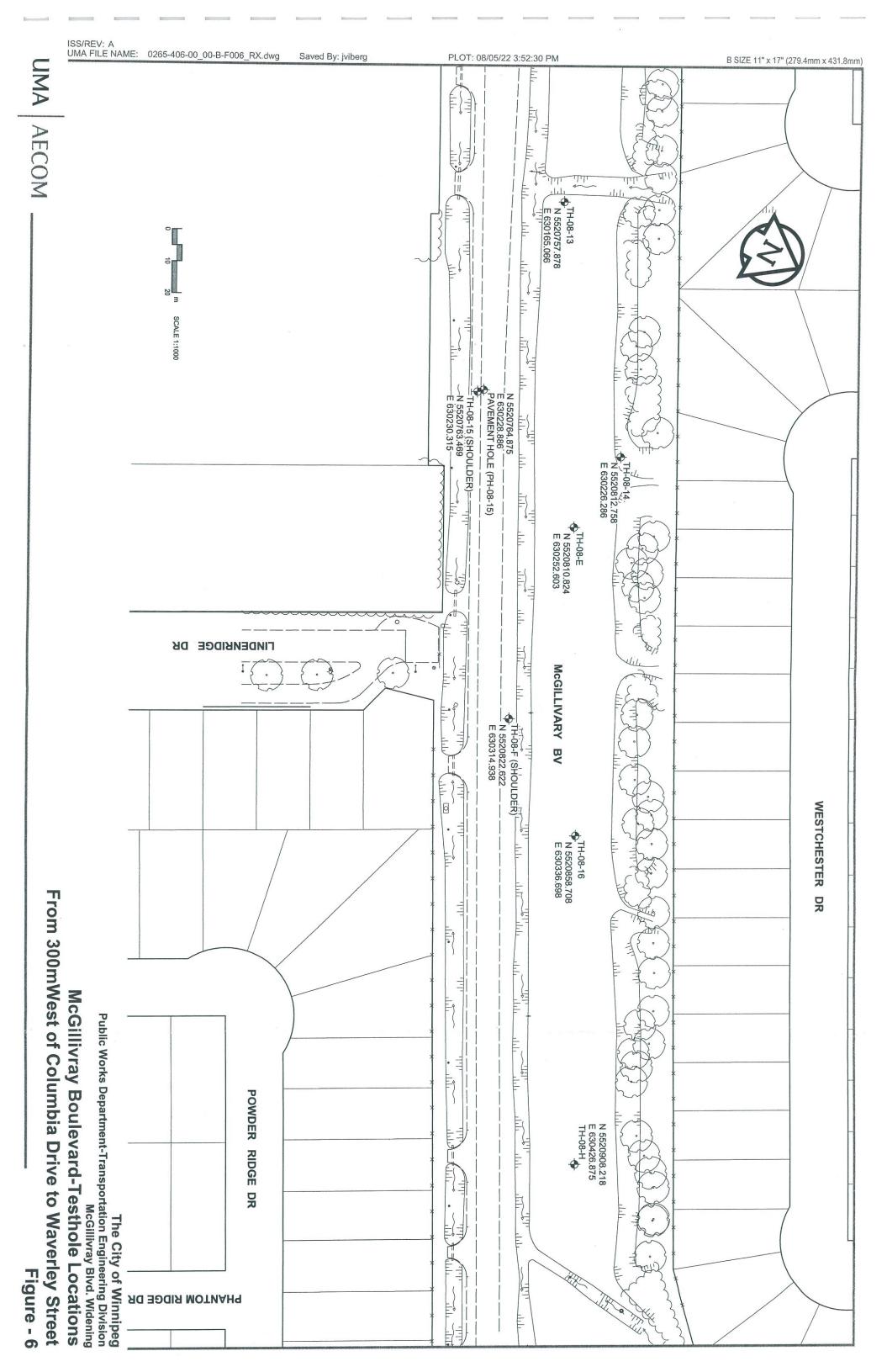


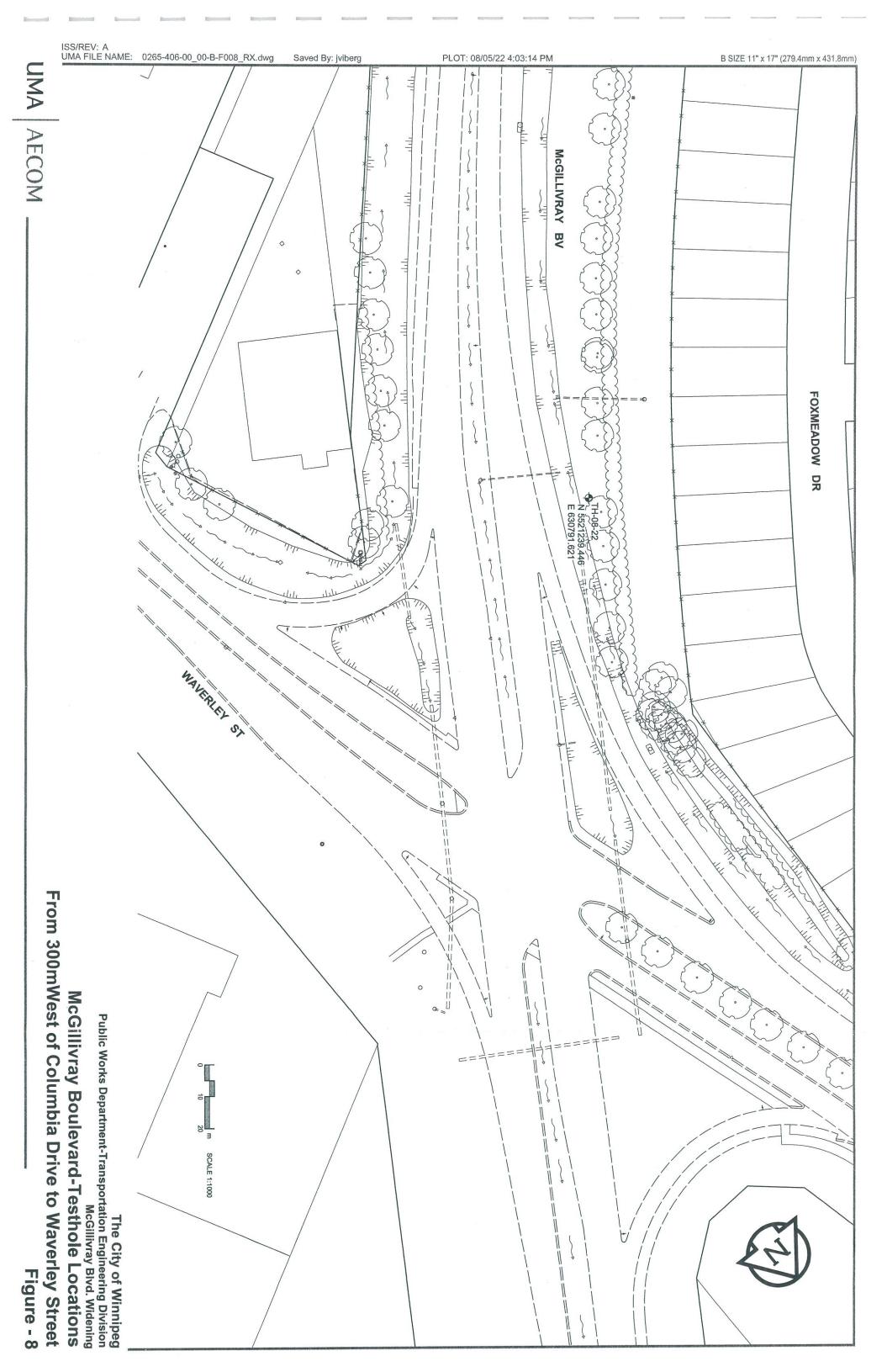












Appendix A
Test Hole Logs

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GENERAL STATEMENT NORMAL VARIABILITY OF SUBSURFACE CONDITIONS

The scope of the investigation presented herein is limited to an investigation of the subsurface conditions as to suitability for the proposed project. This report has been prepared to aid in the evaluation of the site and to assist the engineer in the design of the facilities. Our description of the project represents our understanding of the significant aspects of the project relevant to the design and construction of earth work, foundations and similar. In the event of any changes in the basic design or location of the structures as outlined in this report or plan, we should be given the opportunity to review the changes and to modify or reaffirm in writing the conclusions and recommendations of this report.

The analysis and recommendations presented in this report are based on the data obtained from the borings and test pit excavations made at the locations indicated on the site plans and from other information discussed herein. This report is based on the assumption that the subsurface conditions everywhere are not significantly different from those disclosed by the borings and excavations. However, variations in soil conditions may exist between the excavations and, also, general ground water levels and conditions may fluctuate from time to time. The nature and extent of the variations may not become evident until construction. If subsurface conditions different from those encountered in the exploratory borings and excavations are observed or encountered during construction or appear to be present beneath or beyond excavations, we should be advised at once so that we can observe and review these conditions and reconsider our recommendations where necessary.

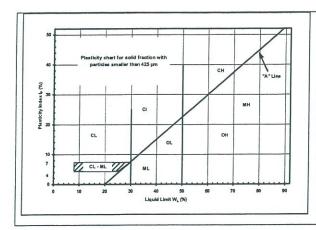
Since it is possible for conditions to vary from those assumed in the analysis and upon which our conclusions and recommendations are based, a contingency fund should be included in the construction budget to allow for the possibility of variations which may result in modification of the design and construction procedures.

In order to observe compliance with the design concepts, specifications or recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated, we recommend that all construction operations dealing with earth work and the foundations be observed by an experienced soils engineer. We can be retained to provide these services for you during construction. In addition we can be retained to review the plans and specifications that have been prepared to check for substantial conformance with the conclusions and recommendations contained in our report.

EXPLANATION OF FIELD & LABORATORY TEST DATA

					UMA	USCS		Laborator	y Classification Crit	егіа
		Descript	tion		Log Symbols	Classification	Fines (%)	Grading	Plasticity	Notes
		CLEAN GRAVELS	sandy grave	ed gravels, els, with little fines	2721	GW	0-5	C _U > 4 1 < C _C < 3		
	GRAVELS (More than 50% of	(Little or no fines)	sandy grave	ded gravels, els, with little fines		GP	0-5	Not satisfying GW requirements		Dual symbols if 5- 12% fines.
STICS	fraction of gravel size)	DIRTY GRAVELS		s, silty sandy vels	HA	GM	> 12		Atterberg limits below "A" line or W _P <4	Dual symbols if above "A" line and
INED SO		(With some fines)		vels, clayey gravels		GC	> 12		Atterberg limits above "A" line or W _P <7	4 <w<sub>P<7</w<sub>
COARSE GRAINED SOILS		CLEAN SANDS	Well grad gravelly san or no	ds, with little	0.0	sw	0-5	C _u > 6 1 < C _c < 3		$C_U = \frac{D_{60}}{D_{10}}$ $C_C = \frac{(D_{30})^2}{D_{10} x D_{60}}$
COA	SANDS (More than 50% of	(Little or no fines)	Poorly grad gravelly sand or no	ds, with little	000	SP	0-5	Not satisfying SW requirements		$C_C = \frac{(D_{30})^2}{D_{10} x D_{60}}$
	coarse fraction of sand size)	DIRTY	Silty s sand-silt		33	SM	> 12		Atterberg limits below "A" line or W _P <4	
		(With some fines)	Clayey sand-clay			sc	> 12		Atterberg limits above "A" line or W _P <7	
	SILTS (Below 'A'	W _L <50	Inorganic s clayey fine s slight pl	sands, with		ML				
	negligible organic content)	W _L >50	Inorganic s plast		Ш	МН				
FINE GRAINED SOILS	CLAYS	W _L <30	Inorganic of clays, sand low plasticity	ly clays of		CL				
	(Above 'A' line negligible organic	30 <wl<50< td=""><td>Inorganic cla clays of r plasti</td><td>medium</td><td></td><td>CI</td><td></td><td></td><td>Classification is Based upon Plasticity Chart</td><td></td></wl<50<>	Inorganic cla clays of r plasti	medium		CI			Classification is Based upon Plasticity Chart	
FINE G	content)	W _L >50	Inorganic cla plasticity,			СН				
	ORGANIC SILTS & CLAYS	W _L <50	Organic s organic silty o plasti	clays of low		OL				
	(Below 'A'	W _L >50	Organic clays of high plasticity			ОН				
Н	GHLY ORGAINIC SOILS		Peat and of organic			Pt		on Post ication Limit	Strong colour or odour, and often fibrous texture	
		Asphalt			Till					
.4		Concrete			edrock ferentiated)				UMA	AECOM
	X	Fill			edrock nestone)					

When the above classification terms are used in this report or test hole logs, the designated fractions may be visually estimated and not measured.



FRAC	CTION	SEIVE S	SIZE (mm)	PERCENTAG	RANGES OF SE BY WEIGHT COMPONENTS	
		Passing	Retained	Percent	Identifier	
Gravel	Coarse	76	19	35-50	and	
Gravei	Fine	19 4.75		35-50	and	
	Coarse	4.75	2.00	20-35	"" or "ou" #	
Sand	Medium	2.00	0.425	20-35	"y" or "ey" *	
	Fine	0.425	0.075	10-20	some	
Silt (nor	-plastic)			10-20	Some	
	(plastic)	< 0.0	75 mm	1-10	trace	

^{*} for example: gravelly, sandy clayey, silty

Definition of Oversize Material

COBBLES: 76mm to 300mm diameter BOULDERS: >300mm diameter

LEGEND OF SYMBOLS

Laboratory and field tests are identified as follows:

qu - undrained shear strength (kPa) derived from unconfined compression testing.

T_v - undrained shear strength (kPa) measured using a torvane

pp - undrained shear strength (kPa) measured using a pocket penetrometer.

L_v - undrained shear strength (kPa) measured using a lab vane.

F_v - undrained shear strength (kPa) measured using a field vane.

γ - bulk unit weight (kN/m³).

SPT - Standard Penetration Test. Recorded as number of blows (N) from a 63.5 kg hammer dropped 0.76 m (free fall) which is required to drive a 51 mm O.D. Raymond type sampler 0.30 m into the soil.

DPPT - Drive Point Pentrometer Test. Recorded as number of blows from a 63.5 kg hammer dropped 0.76 m (free fall) which is required to drive a 50 mm drive point 0.30 m into the soil.

w - moisture content (WL, WP)

The undrained shear strength (Su) of a cohesive soil can be related to its consistency as follows:

Su (kPa)	CONSISTENCY
<12	very soft
12 – 25	soft
25 – 50	medium or firm
50 – 100	stiff
100 – 200	very stiff
200	hard

The resistance (N) of a non-cohesive soil can be related to compactness condition as follows

N - BLOWS/0.30 m	COMPACTNESS
0 - 4	very loose
4 - 10	loose
10 - 30	compact
30 - 50	dense
50	very dense

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Test		Pavement Surface	t Surface	Pavement Structure Material	Aaterial				Moisture		Hydrometer Analysis	Analysis		Atte	Atterberg Limits	
hole	Testhole	Tvne	Thickness (mm)		Thickness (mm)	Subgrade	Start Fnd	Scation	Content (%)	Gravel (%)	Sand	Silt %	Clay (%)	Plastic	Liquid F	Plasticity
TH-08-01	N 5519954.772 E 628740.653					Sand and Gravel (Fill)	0	0.5	,			(21)	H			
						Clay (Fill)	1.5	2	4.8							
						Clay (Fill)	3.5	E 4	24.5						t	
						Clay (Fill)	4.5	5	25.5							
						Clay	5.5	9 1	36.7			1	1	1	1	
TH-08-02	N 5520066.465 E 628916.134					Clav (Fill)	0.5	-	30.7	Ī	Ī	t	T	T	T	T
						Clay (Fill)	1.5	2	33.6							
						Sit	2.5	8	30.1							
						Sil	3.5	4	27.0							
						Clay	U. 7.	0 0	38.6	T				1		Ì
						Clay	6.5	0 ~	41.3					T		
TH-08-03	N 5520110.413 E 628986.182					Sand and Gravel (Fill)	0.5	-								
						Clay (Fill)	2.5	3	31.4							
						Clay (Fill)	3.5	4	33.9	0	4.1	11.3	84.6	26.1	9.96	70.5
						Clav (Fill)	C. 4.	0 6	37.1		Ī		+	1	1	
						Clay		7	45.1							
TH-08-04	N 5520286.549 E 629305.516					Topsoil		9.0	31.6							
						Clay (organic)										
								2	25.9						1	
						Clav	3.5	0 4	39.5		T			T		
						Clay		2	41.0	0	6.0	11.1	88.1	27.6	96.7	69.1
						Clay	1 1	9	42.9							
						Clay		7	44.5							
TH-08-05	N 5520388.913 E 629368.985					Sand and Gravel (Fill)		-	,							
						Clay and Silt (fill)		e -	24.6		Ī			T		
						Clay and Silt (fill)		4 10	35.9							
						Clay	5.5	9	41.8							
	-					Clay	- 1	7	42.4							
90-80-H	N 5520401.764 E 629477.587					Topsoil	- 1	0.5	22.9				1			
						Clay	2.5	7 60	29.3		T		T			
						Clay	3.5	4	30.7							
						Clay	4.5	2	36.6							
						Clay	5.5	9 1	43.7		1				1	
TH 08 07	N 5520307 086 E 620406 034					TosocT	6.0		42.0		1	T	1	1	T	
200	10000000000000000000000000000000000000					Clav (organic)	0.5	-	29.6							
						Clay (organic)	1.5	2	25.2							
						Silt	2.5	8	28.5			1		1		
						Clay	3.5	4 4	33.4			1	1		Ī	
						Clav	5.5	9	45.3							
						Clay	6.5	7	49.3							
TH-08-08	N 5520501.075 E 629654.389					Topsoil	0	0.5	30.4							
						100	2.5	3 6	24.2							
						Clay	3.5	4	34.4							
						Clay	4.5	20	36.0				1		1	
						Clay	6.5	7	43.1							
TH-08-09	N 5520548.686 E 629774.606					Topsoil	-									
						Clay (organic)	0.5	- 0	42.8				1	1		
						Clay	2.5	2 6	35.2			Ì	1			
						Clay	3.5	4	37.5	0	6.0	11.1	88.1	29.9	94.5	64.6
						Clay	4.5	2	39.6							
						Clay	2.5	9 1	43.8			1				
						Sit	0.0		42.0				Ī			
	1					Clay	80	8.5	50.0							
TH-08-10	N 5520526.847 E 629808.035					Sand and Gravel (Fill)	0.5	- "	38.1	00	27	15.7	81.4	24.1	103.4	70.4
						Clay (Fill)	3.5	9	35.0	0.6	4.1	7	1	74.1	1.00	1.0
						Clay (Fill)	4.5	5	32.7							

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2008 Street Renwal Program - McGillivray Boulevard
Geotechnical Investigation

Test		G	Suface to	Domest O terrational	Top con											
hole	acthoral action	Рауеше	Thickness	Pavement Structure Material	Thickness	O. C.	Samula	Sample Location	Moisture	-	Hydrometer Analysis	r Analysis		Atte	ᇎ	
No.	Location	Type	(mm)	Type	(mm)	Description	Start	End	Content (%)	(%)	Sand (%)	# %	(%)	Plastic	Limit	Plasticity
						Clay (Fill)	5.5	9	34.4							
11.00 44						Clay	6.5	7	35.8							
-00-1	N 3320002.200 E 029984.033					Topsoil	, ,									
						Clay	0.5	- (29.9						1	I
						100	2.5	4 6	27 B						1	
						Clav	3.5	4	36.0						T	
						Clay	4.5	2	39.5							
						Clay	5.5	9	41.7							
						Clay	6.5	7	42.5							
TH-08-12	N 5520687.688 E 630004.117					Topsoil	0	0.5	30.8							
						Clay	1.5	2	30.5							
						Silt	2.5	8	24.2							
						Clay	3.5	4	34.4							
						Clay	4.5	2	40.6							
						Clav	5.5	00	411							I
						Clav	8.5	2	48.4						1	
TH-08-13	N 5520757.878 E 630165.066					Topeoil	2	400								I
	L					I Openia		0.0	29.3							
						1100	0.0	7	30.3							
						TIS C	2.5		31.7							
						Silt	3.5	4	25.6							
						Silt	4.5	5	23.3	0	10.7	75.7	13.5	16.6	22.2	5.5
						Silt	5.5	9	35.2							
						Silt	6.5	7	24.0							
						Clay	80	8.5	47.2							
TH-08-14	N 5520812.758 E 630226.286					Topsoil	0	0.5	25.4							
						Topsoil	1.5	2	20.3							
						Sit	2.5	6	22.9							
						Clay	3.5		38.0							I
						Clay	4.5	- 4	34.3						1	I
						Ciay	2 4	0	24.0							
						Clay	0.0	0	47.6							
TH-08-15	N 5520763 469 E 830230 315					Seed and Crim	0.0		0.					1	1	
	1000000 T 00000000					Sand and Gravel (rill)	0 4	0.0							1	
						Clay (Fill)	2.5		27.0							
						Clay (Fill)	3.5	4	24.5							
						Clay	4.5	5	28.6							
						Clay	5.5	9	27.5							
	-					Clay	6.5	7	29.3							
TH-08-16	N 5520858.708 E 630336.698					Topsoil	0	0.5	25.1							
	_					is		-								
						Clav	7.	2	29.4							
						Clav	2.5	6	28.6							I
						Velo	200		30.0	c	00	14 5	9 7 6	28.4	4000	72.2
						Class	2 4	4	30.5	0	0.0	2.	0.4.0	70.1	7.00.7	7.7
						Clay	0.4	0	29.0							
						Sign Sign Sign Sign Sign Sign Sign Sign	0.0	0 1	44.U							I
TIL 00 47	N FEDORAL DOOR T GOOD STOOMS					110	0.0		40.0							
1-00-11	N 3320913.930 E 0304/9.301					Clay (FIII)			- 00						-	0
			-			Clay (Fill)	0.1	7	38.5	0	14.8	40.9	44.3	24.1	8/	53.9
						Clay (FIII)	2.5	2	30.3			(1)				
			1			Clay (Fill)	3.5	4	33.2							
						Clay (Fill)	0.4	0	34.5							
						Clay	5.5	٥	28.6							
						Tion in	6.5	,	38./							
						Clay	_	6.7								
TH-08-18	N 5520952.802 E 630507.542					Topsoil	0	0.5	26.4							
						Clay	1.5	2	32.4							
						Clay	2.5	3	33.5							
	The second secon					Clay	3.5	4	39.1							
						Silt	4.5	2	38.3					100		
						Clay	5.5	9	43.8							
						Clay	6.5	7	48.6							
TH-08-19	N 5520956.737 E 630577.258					Sand and Gravel (Fill)	0	0.5								
	_					Clav (Fill)	2.5	3	37.4							
						Clay (Fill)	3.5	4	34.3							
						Clav	4.5	2	33.9							
						Clay	5.5	9	34.7						The same of the sa	
						Clay	6.5	7	39.0							
TH-08-20	N 5521008.756 E 630565.436					Topsoil	0	0.5	25.4							
						Clay	1.5	2	33.1							
						Clay	2.5	8	42.1							

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1964 1, 144 1,	Test		Pavement	Surface	Davament Structure Material	Material											
Mathematic Mathemati	hole	Testhole		Thickness		Thickness	Subarade	Sample Lo		Content		Sand	Silt	Sel	Plactic	erberg Limi	S
N 552 1076 411 E 600020 169	No	Location		(mm)		(mm)	Description		End	(%)	(%)	(%)	(%)	(%)	Limit	Limit	Index
N 5520100 A11 E GOOD 2 199 20							Clay	П	4	44.5							
N 552 1076 41 E 620075 1924 COMPATION							Clay		2	46.4							
N 552010 6 41E EGODOS 189 N 552010 6 41E EGODOS 1891 N 552011 3 04E EGODOS 1874 N 5							Clay		9 1	48.8		1					
N 55202016 5506 E 6200791 624 Chart (Fill) 1.0	TH-08-21	N 5521076,411 E 630682.198					Sand and Gravel (Eill)	1	40	40.7							
N 552010 524 E 50073 524 Charter of the control o							Clav (Fill)		0.0	10.6							
N 9520510 40E E00073 621 Clay 10							Clay (Fill)		3 8	35.0							
N 16520140 627 E 620075 624 E 6							Clay (Fill)		4	31.8							
N 9520010 2016 E 600701 621 Clay 1							Clay		5	32.0							
N 55201239 440 E 630731 621 N 55201239 440 E 630731 621 N 552019 6 620 7 1 15 15 15 15 15 15 15 15 15 15 15 15 1							Clay		9	36.3							
N 55200404 677 E 620075 202 E 500074 509 N 55200404 677 E 620070 502 H N 55200404 677 E 620070 502 H N 55200404 677 E 6200770 509 E 500 C C C C C C C C C C C C C C C C C C	TH 00 00	T 000 000 000 000 000 000 000 000 000 0					Clay	- 1	7	41.3							
N 8520844 RTP E 6208676 4022 N 8520844 RTP E 6208676 4022 N 8520840 RESTORES E 6200242 6205 N 8520840 RESTORES E 6200246 RPP N 8520840 RPP	11-00-ZZ	N 5521239.446 E 630/91.621					Topsoil		-	29.4							
N 6520464 677 E 628075 402 N 6520464 677 E 628075 402 N 6520464 677 E 628076 653 N 6520464 677 E 628076 653 N 6520464 672 E 620274 653 N 6520464 672 E 620274 653 N 6520462 673 E 620274 673 N 7 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8							Sit		1.5	25.1							
N 6520940 4 677 E 629075 402 N 6520940 877 E 629074 539 N 6520940 877 E 6290							Clay	1	2.5	40.0							
N 5520916 577 E 629975, 402 N 5520916 559 E 629977 502 N 5520916 559 E 629977 502 N 5520916 519 E 629075 524 N 5520917 519 E 629075 524 N 5520916 519 E 629075 524 N							Clay		3.5	43.4							
N 5520404 677 E 623675 402 N 5520404 677 E 623675 402 N 5520404 677 E 6236740 653 N 5520910 824 E 620252 623 N							Clay		4.5	46.9							
N 5520010 21 E 630075 402 Clay Cl							Clay		5.5	50.6							
N 5520922 622 E 620024 875 C 20 C 2	. 00						Clay		6.5	51.4							
Clay 16 2 2	H-08-A	N 5520494.677 E 629675.402					Topsoil										
Clay Size							Clay		-	19.8							
Silit Sign Silit Sign Silit Sign Silit Sign Sig							Clay		2	23.2							
Clay 5.5 5.5 N 5620016 16.356 E E20007 5.024 5.5 5.5 N 5620010 16.356 E E30075 524 5.5 N 5620010 2.16 E E30075 524 5.5 N 5620010 2.16 E E30076 677 5.5 N 5650010 2.16 E E30076 7.0 N 5650010 2							Silt		3	18.4							
Clay A5 S							Clay		4	30.8							
Clay 6.5 6							Clay		2	34.3							
N 5520616.359 E 620897 830 Clay 6.5 7							Clay		9	36.5							
N 5520616.359E 623997.830 N 5520713.048 E 630075.924 N 5520810.824 E 630075.924 N 5520810.824 E 630025.603 N 552082 E 6300214.836 N 552080.821 E 630426.875 N 552080.821 E 6300714.875 N 552080.821							Clay	П	7	40.4							
N 5520871 0AB E 630075 924 Clay (Ciganic) 15 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	TH-08-C	N 5520616.359 E 629897.830					Topsoil	ı									
N 5520910 524 E 630075 924 N 5520909 218 E 630075 924 N 5520909 218 E 630075 979 N 5520909 218 N 5520909 218 E 630075 979 N 5520909 218 N 5520909 218 E 630075 979 N 5520909 218 N 5520909							Silt		-	28.8							
N 5520922 622 E 630426 875 N 5520908 216 E 630426 875 N							Clay (organic)		2	30.5							
N 5520910 218 E 630075.924 Clay 6.5 C Clay 7 C							Clay (organic)		3	28.9							
Clay 45 5 N 65209713.046 E 630075.924 1 N 6520910.624 E 630076.924 1 N 6520910.624 1 N 6520910.624 1 N 6520910.624 1							Clay		4	31.5							
Clay S.S.							Clay		2	39.2							
N 5520713,048 E 630075,924							Clay		9	43.3							
N 5520613,048 E 630075,924 SIR 15 2 2 SIR 1 15 SIR 1							Clay		7	43.8							
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N 55209022.022 E 630314.938						(Silt		3	25.7							
Clay 6.5 6							Clay		4	36.5							
Sign							Clay		5	34.3							
Sitt Glay						/	Clay		9	37.6							
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N 5520810.824 E 630252.603 Clay C							Clay	l									
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Clay 1.5 2							Silt			22.3							
Clay 2.5 3	Š						Clay		2	22.3							
Clay 3.5 4							Clay		3	28.6							
Clay 4.5 5							Clay		4	38.4							
Clay 5.5 6							Clay		5	38.6							
Clay 6.5 7							Clay		9	42.3							
N 5520622,622 E 630314,936 N 5520622,622 E 630314,936 N 5520622,622 E 630314,936 N 5520908,218 E 630426,875 N 5520908,218 E 630426,875 N 5520908,218 E 630426,875 N 5519954,772 E 628740,653 N 5519954,772 E 628740,653 N 5519954,772 E 628740,653 N 5519954,772 E 628740,653 N 5520908,218 E 630314,936 N 5519954,772 E 628740,653							Clay		7	38.0							
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N 5520822,622 E 630314,938 Sand and Gravel (Fill) 0.5 1 N 5520808.218 E 630426.875 Clay (Fill) 2.5 3 N 5520908.218 E 630426.875 Clay (Fill) 3.5 4 N 5520908.218 E 630426.875 Clay 6.5 7 N 5520908.218 E 630426.875 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7							Clay	- 1	8.5	49.1							
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N 5520908,218 E 630426,875							Clay (Fill)		е.	30.5							
N 5520908.218 E 630426.875 Clay Clay 6.5 7 N 5520908.218 E 630426.875 Clay 6.5 7 Clay 2.5 3 Clay 3.5 4 Clay 4.5 6 Clay 4.5 6 Clay 6.5 7 N 5519954.772 E 628740.653 Clay 6.5 7 N 5519954.772 E 628740.653 Clay 6.5 7 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 6 Clay 6.5 7 Clay 6.5 7 Clay 6.5 7 Clay 6.5 7 Clay 6.5 6 Clay 6.5 7 Clay 7 Clay 6.5 7 Clay 7 Clay 6.5 7 Clay 7 Clay 6.5 7 Clay 6.5 7 Clay 7 Clay 7 Clay 7 Clay 6.5 7 Clay 7 Clay 6.5 7 Clay 7 C							Clay (FIII)		4	7.77							
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N 5520908.218 E 630428.875 Clay Clay 2.5 Silt 1.5 2 Silt 1.5 2 Silt 1.5 2 Clay 2.5 3 Clay 2.5 3 Clay 2.5 5 Clay 2.5 5 Clay 2.5 5 Clay 2.5 7 Clay 2.5 6 Clay 3.5 6 Clay 4.5 5 Clay 4.5 5 Clay 4.5 5 Clay 4.5 6 Clay 4.5 5 Clay 4.5 5 Clay 4.5 6 Clay 4.5 7 Clay 4.5 6 Cla							Oliver College		2 1	33.0							
Signature Color	TH-08-H	N 5520908 218 E 630426 875					Tongil		90	20.00	T	I					
N 5519954,772 E 628740,653 SIR Clay Clay Clay Clay Clay Clay Clay Clay		14 3020303.10 E 030420.013					Ilopsoli Sile		0.0	23.3							
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Clay 0.5 1	TH-08-1	N 5519954.772 E 628740.653					Topsoil										
1.5							Ne C		-	26.0							
-							Sill		2	17.8							

UMA AECOM

City of Winnipeg 2008 Street Renwal Program - McGillivray Boulevard Geotechnical Investigation

Plasticity Index Atterberg Limits Liquid Plastic Limit Clay (%) Hydrometer Analysis Silt (%) Sand (%) Gravel % (%) 38.0 39.3 42.0 41.9 42.5 Moisture Content Sample Location Start 2.5 3.5 4.5 5.5 6.5 Subgrade
Description
Silt
Clay
Clay
Clay
Clay
Clay Pavement Structure Material
Thickness (mm) 215 150 Pavement Surface Thickness PH-08-10 N 5520536.242 E 629803.510 Concrete 190 115 (mm) 175 190 Concrete PH-08-03 N 5520121.741 E 628999.844 Asphalt PH-08-05 N 5520387.296 E 629366.765 Asphalt PH-08-15 N 5520764.875 E 630228.886 Concrete PH-08-17 N 5520911.549 E 630480.683 Concrete PH-08-19 N 5520959.239 E 630575.690 Concrete PH-08-01 N 5519952.636 E 628742.122 Asphalt Type N 5521071.265 E 630689.325 Testhole Location PH-08-21 Test hole No.

PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg TESTHOLE NO: TH-08-01 PROJECT NO.: 0265-40600-2100 LOCATION: N 5519954.772 E 628740.653 ELEVATION (m): 233.98 CONTRACTOR: Paddock Drilling Ltd. METHOD: CT-250, 150mm SSA NO RECOVERY CORE SPLIT SPOON BULK MSHELBY TUBE SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ ELEVATION SOIL SYMBOL DEPTH (m) ◆ SPT (Standard Pen Test) ◆ SAMPLE Z (Blows/300mm)
0 40 60 80 100

Total Unit Wt (kN/m³)
7 18 19 20 21 SOIL DESCRIPTION COMMENTS 40 60 SAND AND GRAVEL (Fill) 0 - dark brown G36 - dense, dry to moist - fine grained sand to coarse grained gravel poorly graded, sub angular CLAY (Fill) - some silt, trace sand, trace gravel, trace organics - dark brown and black - frozen - high plasticity G37 - trace sand below 0.8 m G38 233 G39 G40 CLAY - some silt, trace sand - mottled brown and dark brown - frozen - high plasticity G41 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 232 G42 END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. Seepage at 0.3 m from sand and gravel fill. 3. Test hole backfilled with auger cuttings to 0.9 m and bentonite chips to ground surface. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 4/28/08 UMA AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: TH-08-02 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg PROJECT NO.: 0265-40600-210 LOCATION: N 5520066.465 E 628916.134 ELEVATION (m): 233.788 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE BULK SHELBY TUBE SPLIT SPOON SAMPLE TYPE GRAB PENETRATION TESTS SAMPLE TYPE ELEVATION SAMPLE # SOIL SYMBOL DEPTH (m) SPT (N) (Blows/300mm) 40 60 80 100 COMMENTS SOIL DESCRIPTION ■ Total Unit Wt ■ (kN/m³) CLAY (Fill) - some silt, trace sand, trace gravel, trace organics - black - stiff, moist - high plasticity G200 G201 SILT - some clay, trace sand - brown - soft, wet 233 - low plasticity G202 G203 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.) - mottled brown and dark brown - firm, moist - high plasticity G204 G205 232 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G206 END OF TEST HOLE AT 2.3 m IN CLAY 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 231 COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 REVIEWED BY: N. Ferreira UMA AECOM Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

TESTHOLE NO: TH-08-03 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520110.413 E 628986.182 ELEVATION (m): 233.924 CONTRACTOR: Paddock Drilling Ltd. METHOD: CT-250, 150mm SSA NO RECOVERY CORE BULK MSHELBY TUBE SPLIT SPOON SAMPLE TYPE GRAB PENETRATION TESTS ★ Becker ★ SAMPLE TYPE SYMBOL ♦ Dynamic Cone ♦ **ELEVATION** SAMPLE # DEPTH (m) ◆ SPT (Standard Pen Test) ◆
(Blows/300mm)
0 20 40 60 80 100 SOIL DESCRIPTION COMMENTS ■ Total Unit Wt SOIL (kN/m³) 18 19 60 -l₈₀ SAND AND GRAVEL (Fill) - dark brown, dense, dry to moist 0 - fine grained sand to coarse grained gravel - poorly graded, sub angular CLAY (Fill) - some silt, trace sand, trace organics, dark brown and black, frozen, high plasticity G29 G30 G31 233 **GRAIN SIZE** DISTRIBUTION: G32 **CLAY 84.6%** SILT 11.3% **SAND 4.1%** G33 G34 SILT - some clay, trace sand, trace organics - brown 232 - soft, moist, low plasticity CLAY - some silt, trace sand -2 **UMA WINN.GDT** G35 mottled brown and dark brown firm, moist high plasticity END OF TEST HOLE AT 2.3 m IN CLAY MCGILLIVRAYBLVD.GPJ Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. LOG OF TEST HOLE THLOGS 231 LOGGED BY: R. Belbas COMPLETION DEPTH: 2.29 m COMPLETION DATE: 4/28/08 **AECOM** REVIEWED BY: N. Ferreira UMA PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: TH-08-04 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg LOCATION: N 5520286.549 E 629305.516 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 233.167 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE BULK MSHELBY TUBE SPLIT SPOON GRAB SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL ELEVATION SAMPLE # SPT (Standard Pen Test) DEPTH (m) (Standard Feb. (Blows/300mm) 60 80 100 2 COMMENTS SOIL DESCRIPTION 0 40 60 8

Total Unit Wt (kN/m³) 18 19 TOPSOIL 0 G193 233 CLAY (Organic) - silty, trace sand - black - stiff, moist high plasticity SILT - some clay, trace sand G194 - brown - soft, moist - low plasticity G195 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.) - dark brown - stiff, moist high plasticity G196 232 **GRAIN SIZE** DISTRIBUTION: G197 CLAY 88.1% - firm below 1.4 m SILT 11.1% **SAND 0.9%** G198 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G199 231 END OF TEST HOLE AT 2.3 m IN CLAY Notes: No seepage or sloughing. 2. Test hole backfilled with bentonite chips and auger cuttings. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 REVIEWED BY: N. Ferreira UMA AECOM Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-05 CLIENT: City of Winnipeg LOCATION: N 5520388.913 E 629368.985 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 233.304 METHOD: CT-250, 150mm SSA SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE SAMPLE TYPE GRAB PENETRATION TESTS SAMPLE TYPE SYMBOL ELEVATION DEPTH (m) SPT (Standard Pen Test) SAMPLE # (Blows/300mm) 40 60 80 100 SOIL DESCRIPTION COMMENTS SPT (Total Unit Wt (kN/m³) SOIL 19 SAND AND GRAVEL (Fill) 0 - dark brown - dense, dry to moist fine grained sand to coarse grained gravel
poorly graded, sub angular G22 233 CLAY AND SILT (Fill) - trace sand, trace gravel, trace organics - brown and black - stiff, moist - intermediate to high plasticity G23 G24 G25 232 G26 CLAY - some silt, trace sand - brown stiff, moist - high plasticity G27 THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G28 231 END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. LOG OF TEST HOLE COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas UMA AECOM COMPLETION DATE: 4/28/08 REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-06 CLIENT: City of Winnpeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520401.764 E 629477.587 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.636 METHOD: CT-250, 150mm SSA NO RECOVERY CORE SPLIT SPOON BULK SHELBY TUBE SAMPLE TYPE PENETRATION TESTS * Becker * SAMPLE TYPE ♦ Dynamic Cone ♦ ELEVATION SOIL SYMBOL SPT (Standard Pen Test) DEPTH (m) SAMPLE # (Blows/300mm) 40 60 80 100 SOIL DESCRIPTION **COMMENTS** SPT (■ Total Unit Wt ■ (kN/m³) TOPSOIL 0 G64 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.) - dark brown - stiff, moist - high plasticity frozen from 0.6 to 1.4 m G65 232 G66 G67 G68 firm below 1.4 m 231 G69 SILT - some clay, trace sand G70 UMA WINN.GDT - light brown - soft, moist, low plasticity CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.) - dark brown - stiff, moist, high plasticity END OF TEST HOLE AT 2.3 m IN CLAY LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ Notes: 1. No seepage or sloughing. Test hole backfilled with auger cuttings. 230 LOGGED BY: R. Belbas COMPLETION DEPTH: 2.29 m COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg TESTHOLE NO: TH-08-07 LOCATION: N 5520397.086 E 629496.934 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.555 METHOD: YANMAR C25R, 150mm SSA SPLIT SPOON BULK NO RECOVERY CORE MSHELBY TUBE SAMPLE TYPE GRAB PENETRATION TESTS SAMPLE TYPE SOIL SYMBOL ELEVATION DEPTH (m) SAMPLE # SPT (Standard Pen Test) Z (Blows/300mm) 0 40 60 80 100 Total Unit Wt (kN/m³) SOIL DESCRIPTION COMMENTS TOPSOIL CLAY (Organic) - some silt, trace sand - black - stiff, moist G186 - high plasticity 232 - firm below 0.5 m SILT - some clay, trace sand - brown - firm, moist - low plasticity G188 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.) - dark brown - firm, moist high plasticity G189 G190 231 G191 soft below 1.8 m OG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G192 END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with bentonite chips and auger cuttings. 230 COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas UMA AECOM REVIEWED BY: N. Ferreira COMPLETION DATE: 5/5/08 PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-08 CLIENT: City of Winnpeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520501.075 E 629654.389 ELEVATION (m): 232.908 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA BULK NO RECOVERY CORE SPLIT SPOON MSHELBY TUBE SAMPLE TYPE GRAB PENETRATION TESTS SAMPLE TYPE ELEVATION SOIL SYMBOL SAMPLE # DEPTH (m) SPT (Standard Pen Test) Z (Blows/300mm)
0 40 60 80 100

Total Unit Wt (kN/m³)
7 18 19 20 21 SOIL DESCRIPTION COMMENTS TOPSOIL 0 G172 SILT - some clay, trace sand - brown - soft, moist - low plasticity G173 G174 232 CLAY - some silt, trace sand mottled brown and dark brown firm, moist high plasticity G175 G176 G177 231 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G178 END OF TEST HOLE AT 2.3 m IN CLAY Notes: No seepage or sloughing.
 Test hole backfilled with auger cuttings. 230 COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

TESTHOLE NO: TH-08-09 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg LOCATION: N 5520548.686 E 629774.606 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 233.042 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA CORE NO RECOVERY BULK MSHELBY TUBE SPLIT SPOON SAMPLE TYPE GRAB PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦
SPT (Standard Pen Test) ♦ SYMBOL ELEVATION SAMPLE # DEPTH (m) Z (Blows/300mm) 0 40 60 80 100 SOIL DESCRIPTION COMMENTS SOIL (kN/m³) 18 19 18 TOPSOIL 233 0 CLAY (Organic) - silty, trace sand, trace rootlets G164 - black firm, moist Intermediate to high plasticity CLAY - some silt, trace sand, trace organics G165 - dark brown - stiff, moist - high plasticity G166 232 **GRAIN SIZE** DISTRIBUTION: G167 **CLAY 88.1%** firm below 1.1 m SILT 11.1% **SAND 0.9%** G168 G169 231 G170 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT SILT - some clay, trace sand - brown - soft, wet, low plasticity CLAY - some silt, trace sand - dark brown - firm, moist - high plasticity G171 COMPLETION DEPTH: 3.51 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 **UMA AECOM** REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 2

PROJ	ECT:	McGillivray Boulevard Widening	CLIENT: Cit	ty of Winns	oeg				TESTH	HOLE NO: TH-08-09)
		N 5520548.686 E 629774.606		Sterring					PROJE	ECT NO.: 0265-406	00-210
CONT	RAC	TOR: Paddock Drilling Ltd.	METHOD: Y				50mr	n SSA		TION (m): 233.042	
SAMF			SPLIT SPOO		BL			☑ NO R	ECOVERY	CORE	
DEPTH (m)	SOIL SYMBOL	SOIL DESCRIPTION		ר היירים ויירים	SAMPLE LYPE	SAMPLE #	SP	PENETRATION TE # Becker # Opynamic Con SPT (Standard Per (Blows/300mn 2 0 40 60 Total Unit W (kN/m) 16 17 18 19 Plastic MC 20 40 60	e 🔷	COMMENTS	ELEVATION
3						1					230 —
TEST HOLE I HLUGS_MCGILLIWRAYBLVD.GPJ UMA WINN.GDJ S/22/08		END OF TEST HOLE AT 3.5 m IN CLAY Notes: 1. Seepage at 2.1 m from silt layer. 2. No sloughing. 3. Test hole backfilled with bentonite chips and auger cuttings.									229 —
6				LOGGED	BY:	R. E	Belbas	3	COMPLET	TION DEPTH: 3.51 m	
		UMA AECOM		REVIEWE	D BY	Y: N	. Ferr	eira	COMPLET	TION DATE: 5/5/08	
000		1		PROJECT	EN	GINE	ER:	Nelson Ferreira		Page	2 of 2

PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg TESTHOLE NO: TH-08-10 PROJECT NO.: 0265-40600-2100 LOCATION: N 5520526.847 E 629808.035 ELEVATION (m): 233.546 CONTRACTOR: Paddock Drilling Ltd. METHOD: CT-250, 150mm SSA BULK CORE NO RECOVERY SHELBY TUBE SPLIT SPOON SAMPLE TYPE GRAB PENETRATION TESTS ★ Becker
 ★
 ◆ Dynamic Cone ◆ SAMPLE TYPE ELEVATION SOIL SYMBOL DEPTH (m) SAMPLE # SPT (Standard Pen Test) 2 (Blows/300mm)
0 40 60 80 100

Total Unit Wt (kN/m³) SOIL DESCRIPTION COMMENTS 0 SAND AND GRAVEL (Fill) - dark brown - dense, moist - fine grained sand to coarse grained gravel - poorly graded, sub angular G43 CLAY (Fill) - some silt, trace sand, trace gravel, trace organics - dark brown and black G44 233 - firm, moist to 0.6 m, frozen below 0.6 m - high plasticity **GRAIN SIZE DISTRIBUTION:** CLAY 81.4% G45 SILT 15.7% **SAND 2.7%** GRAVEL 0.2% G46 G47 232 G48 CLAY - some silt, trace sand - brown LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 - frozen to 2.0 m, stiff, moist below 2.0 m - high plasticity G49 END OF TEST HOLE AT 2.3 m IN CLAY 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 231 COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 4/28/08 UMA | AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-11 CLIENT: City of Winnpeg LOCATION: N 5520662.266 E 629984.633 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 233.006 METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE SPLIT SPOON BULK MSHELBY TUBE SAMPLE TYPE GRAB PENETRATION TESTS Ж Becker

Ж SAMPLE TYPE ◇ Dynamic Cone ◇
 ◆ SPT (Standard Pen Test) ◆ SOIL SYMBOL ELEVATION SAMPLE # DEPTH (m) Z (Blows/300mm)
0 40 60 80 100 SOIL DESCRIPTION COMMENTS (kN/m³) Plastic MC TOPSOIL CLAY - some silt, trace sand, trace gravel, trace organics - dark brown and black stiff, moist G150 high plasticity G151 SILT - some clay, trace sand - brown - soft, moist - low plasticity - clayey and dark brown below 0.7 m G152 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.) mottled brown and dark brown 232 firm, moist high plasticity G153 G154 G155 5/22/08 231 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G156 soft below 2.1 m END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas REVIEWED BY: N. Ferreira COMPLETION DATE: 5/5/08 UMA AECOM PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: TH-08-12 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg PROJECT NO.: 0265-40600-210 LOCATION: N 5520687.688 E 630004.117 ELEVATION (m): 233.095 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA CORE NO RECOVERY MSHELBY TUBE SPLIT SPOON BULK GRAB SAMPLE TYPE PENETRATION TESTS **※** Becker **※** SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL **ELEVATION** SAMPLE # DEPTH (m) SPT (Standard Pen Test) $\widehat{\mathbf{z}}$ (Blows/300mm) 0 40 60 80 100 SOIL DESCRIPTION COMMENTS SPT (kN/m³) 18 19 H_{80 100} 60 TOPSOIL G143 233 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.) - mottled brown and dark brown - stiff, moist, - high plasticity G144 SILT - some clay, trace sand - brown - soft, moist - low plasticity G145 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.), - mottled brown and dark brown - firm, moist, 232 - high plasticity G146 G147 G148 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G149 231 END OF TEST HOLE AT 2.3 m IN CLAY 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 REVIEWED BY: N. Ferreira UMA AECOM Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

TESTHOLE NO: TH-08-13 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg LOCATION: N 5520757.878 E 630165.066 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.635 METHOD: YANMAR C25R, 150mm SSA CORE NO RECOVERY SPLIT SPOON BULK GRAB MSHELBY TUBE SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL **ELEVATION** SAMPLE # DEPTH (m) SPT (Standard Pen Test) Z (Standard Peri 1931) 4 (Blows/300mm) 40 60 80 100 SOIL DESCRIPTION COMMENTS 0 40 60 8

Total Unit Wt (kN/m³) 18 19 18 60 20 F TOPSOIL (300 mm) 0 G128 SILT - some clay, some sand - brown - firm, moist - low plasticity G129 232 G130 - soft below 0.9 m G131 **GRAIN SIZE** DISTRIBUTION: G132 CLAY 13.5% SILT 75.7% SAND 10.7% 231 G133 - wet below 1.8 m LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G134 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.) G135 - dark brown - firm, moist - high plasticity 230 COMPLETION DEPTH: 3.05 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 2

PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg TESTHOLE NO: TH-08-13 LOCATION: N 5520757.878 E 630165.066 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.635 METHOD: YANMAR C25R, 150mm SSA SHELBY TUBE SPLIT SPOON BULK NO RECOVERY CORE SAMPLE TYPE GRAB PENETRATION TESTS SAMPLE TYPE SOIL SYMBOL ELEVATION SAMPLE # DEPTH (m) \widehat{z} (Blows/300mm)
20 40 60 80 100

Total Unit Wt (kN/m³) SOIL DESCRIPTION **COMMENTS** 18 19 Liquid 60 80 Plastic MC 3 END OF TEST HOLE AT 3.0 m IN CLAY 1. No seepage at 1.8 m from silt layer. 2. No sloughing. 3. Test hole backfilled with bentonite chips and auger cuttings. 229 228 LOG OF TEST HOLE THLOGS MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 227 LOGGED BY: R. Belbas COMPLETION DEPTH: 3.05 m UMA AECOM COMPLETION DATE: 5/5/08 REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 2 of 2

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-14 CLIENT: City of Winnpeg LOCATION: N 5520812.758 E 630226.286 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.712 METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE MSHELBY TUBE SPLIT SPOON BULK SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦

SPT (Standard Pen Test) ♦ SOIL SYMBOL ELEVATION SAMPLE # DEPTH (m) 2 (Blows/300mm) 0 40 60 80 100 Total Unit Wt SOIL DESCRIPTION COMMENTS (kN/m³) 18 19 TOPSOIL G113 G114 SILT - some clay, trace sand 232 - soft, moist, low plasticity - wet below 0.8 m G115 CLAY - some silt, trace sand, trace silt inclusions (< 10 mm dia.) - mottled brown and dark brown - stiff, moist high plasticity G116 G117 - firm below 1.4 m 231 G118 THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G119 END OF TEST HOLE AT 2.3 m IN CLAY 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 230 LOG OF TEST HOLE LOGGED BY: R. Belbas COMPLETION DEPTH: 2.29 m UMA AECOM REVIEWED BY: N. Ferreira COMPLETION DATE: 5/5/08 PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg TESTHOLE NO: TH-08-15 LOCATION: N 5520763.469 E 630230.315 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 233.344 METHOD: CT-250, 150mm SSA BULK MSHELBY TUBE SPLIT SPOON NO RECOVERY CORE GRAB SAMPLE TYPE PENETRATION TESTS SAMPLE TYPE SYMBOL ELEVATION SAMPLE # DEPTH (m) SPT (Standard Pen Test) 2 (Blows/300mm) 0 40 60 80 100 Total Unit Wt (kN/m²) SOIL DESCRIPTION COMMENTS SAND AND GRAVEL (Fill) G50 - dark brown - dense, dry to moist fine grained sand to coarse grained gravel
 poorly graded, sub angular CLAY (Fill) - some silt, trace sand, trace gravel, trace organics - dark brown and black 233 - firm, moist to 0.9 m, frozen below 0.9 m - high plasticity G51 G52 G53 CLAY - some silt, trace sand - frozen to 2.0 m, stiff, moist below 2.0 m 232 - high plasticity G54 G55 THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G56 END OF TEST HOLE AT 2.3 m IN CLAY 231 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. LOG OF TEST HOLE COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 4/28/08 UMA AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-16 CLIENT: City of Winnpeg LOCATION: N 5520858.708 E 630336.698 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.751 METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE SAMPLE TYPE GRAB SHELBY TUBE SPLIT SPOON BULK PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL **ELEVATION** SAMPLE # DEPTH (m) SPT (Standard Pen Test) (Blows/300mm) 40 60 80 100 SOIL DESCRIPTION **COMMENTS** SPT ■ Total Unit Wt (kN/m³) 60 TOPSOIL G106 SILT - some clay, trace sand, trace gravel - brown, soft, moist - low plasticity CLAY - some silt, trace sand mottled brown and dark brown G107 firm, moist - high plasticity 232 G108 **GRAIN SIZE** 100.DISTRIBUTION: G109 CLAY 84.6% SILT 14.5% **SAND 0.9%** G110 soft below 1.4 m G111 231 SILT - some clay, trace sand - brown (oxidized) THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G112 - soft, moist, low plasticity CLAY - some silt, trace sand - mottled brown and dark brown - stiff, moist, high plasticity END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 230 LOG OF TEST HOLE LOGGED BY: R. Belbas COMPLETION DEPTH: 2.29 m UMA AECOM REVIEWED BY: N. Ferreira COMPLETION DATE: 5/5/08 PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: TH-08-17 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg LOCATION: N 5520913.936 E 630479.361 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 233.302 CONTRACTOR: Paddock Drilling Ltd. METHOD: CT-250, 150mm SSA NO RECOVERY CORE BULK SHELBY TUBE SPLIT SPOON GRAB SAMPLE TYPE PENETRATION TESTS ※ Becker

※ SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL ELEVATION SAMPLE # SPT (Standard Pen Test) DEPTH (m) Z (Blows/300mm) 0 40 60 80 100 COMMENTS SOIL DESCRIPTION (kN/m³) 18 MC 40 60 SAND AND GRAVEL (Fill) 0 - dark brown - dense, moist G8 - fine grained sand to coarse grained gravel - poorly graded, sub angular 233 CLAY AND SILT (Fill) - some sand, trace gravel, trace organics - dark brown and black - firm, moist to 0.6 m, frozen below 0.6 m **GRAIN SIZE** - high plasticity DISTRIBUTION: G9 **CLAY 44.3%** SILT 40.9% **SAND 14.8%** G10 G11 232 G12 CLAY - some silt, trace sand - brown frozen high plasticity G13 SILT - some clay, sandy - brown 5/22/08 - soft, moist - low plasticity -2 THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G14 CLAY - some silt, trace sand - mottled brown and dark brown - stiff, moist, high plasticity 231 END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. OF TEST HOLE COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 4/28/08 UMA AECOM REVIEWED BY: N. Ferreira Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-18 CLIENT: City of Winnpeg LOCATION: N 5520952.802 E 630507.542 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.46 METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE SPLIT SPOON BULK GRAB MSHELBY TUBE SAMPLE TYPE PENETRATION TESTS Ж Becker

Ж SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL ELEVATION SAMPLE # DEPTH (m) SPT (Standard Pen Test) (Standard Pen (Blows/300mm) 60 80 100 2 SOIL DESCRIPTION COMMENTS 0 40 60 8

Total Unit Wt (kN/m³) 18 19 20 F 40 TOPSOIL G92 CLAY - some silt, trace sand, trace organics dark brown and black stiff, moist 232 high plasticity G93 mottled brown and dark brown below 0.6 m G94 - firm below 0.8 m G95 SILT - some clay, trace sand, trace gravel - brown G96 231 - soft, moist to wet - low plasticity CLAY - some silt, trace sand G97 - mottled brown and dark brown - firm, moist high plasticity 5/22/08 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G98 - soft below 2.1 m END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 230 2. Test hole backfilled with auger cuttings. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: TH-08-19 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520956.737 E 630577.258 ELEVATION (m): 233.864 CONTRACTOR: Paddock Drilling Ltd. METHOD: CT-250, 150mm SSA BULK NO RECOVERY CORE MSHELBY TUBE SPLIT SPOON SAMPLE TYPE GRAB PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL ELEVATION SAMPLE # DEPTH (m) SPT (Standard Pen Test) (Blows/300mm)
20 40 60 80 100

■ Total Unit Wt ■
(kN/m³)
7 18 19 20 21 Z SOIL DESCRIPTION COMMENTS SPT SAND AND GRAVEL (Fill) - dark brown, dense, moist 0 G57 - fine grained sand to coarse grained gravel - poorly graded, sub angular CLAY (Fill) - some silt, trace sand, trace gravel, trace organics - dark brown and black - stiff, moist to 0.6 m, frozen below 0.6 m - high plasticity G58 G59 233 G60 CLAY - some silt, trace sand - frozen to 2.0 m, stiff, moist below 2.0 m - high plasticity G61 G62 232 THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G63 END OF TEST HOLE AT 2.3 m IN CLAY 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 231 LOG OF TEST HOLE LOGGED BY: R. Belbas COMPLETION DEPTH: 2.29 m COMPLETION DATE: 4/28/08 REVIEWED BY: N. Ferreira UMA AECOM PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg TESTHOLE NO: TH-08-20 LOCATION: N 5521008.756 E 630565.436 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.471 METHOD: YANMAR C25R, 150mm SSA CORE SPLIT SPOON BULK NO RECOVERY SHELBY TUBE SAMPLE TYPE GRAB PENETRATION TESTS SAMPLE TYPE ELEVATION SOIL SYMBOL SAMPLE # DEPTH (m) ◆ SPT (Standard Pen Test) ◆ 2 (Blows/300mm) 0 40 60 80 100 ■ Total Unit Wt ■ (kN/m³) SOIL DESCRIPTION COMMENTS TOPSOIL G78 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.), trace sulfide inclusions (< 5 mm dia.), trace organics - dark brown - stiff, moist high plasticity 232 -G79 firm below 0.6 m G80 - soft below 0.8 m G81 G82 231 G83 LOG OF TEST HOLE_THLOGS_MCGILLIVRAYBLVD.GPJ_UMA_WINN.GDT_5/22/08 G84 END OF TEST HOLE AT 2.3 m IN CLAY 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 230 -LOGGED BY: R. Belbas COMPLETION DEPTH: 2.29 m COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

TESTHOLE NO: TH-08-21 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5521076.411 E 630682.198 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 233.688 METHOD: CT-250, 150mm SSA CORE BULK NO RECOVERY MSHELBY TUBE SPLIT SPOON SAMPLE TYPE PENETRATION TESTS SAMPLE TYPE **ELEVATION** SOIL SYMBOL SAMPLE # DEPTH (m) SPT (Standard Pen Test) (Blows/300mm) 40 60 80 100 SOIL DESCRIPTION **COMMENTS** SPT (Total Unit Wt (kN/m³) 18 19 SAND AND GRAVEL (Fill) - trace to some silt 0 G1 - brown dense, moist - fine grained sand to coarse grained gravel - poorly graded, sub angular CLAY (Fill) - some silt, trace sand, trace gravel, trace organics - dark brown and black - stiff, moist to 0.6 m, frozen below 0.6 m G2 - high plasticity 233 G3 G4 CLAY - some silt, trace sand G5 - frozen to 2.0 m, stiff, moist below 2.0 m high plasticity 232 G6 5/22/08 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G7 END OF TEST HOLE AT 2.3 m IN CLAY Notes: No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 231 -COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 4/28/08 UMA AECOM REVIEWED BY: N. Ferreira Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-22 CLIENT: City of Winnpeg LOCATION: N 5521239.446 E 630791.621 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.23 METHOD: YANMAR C25R, 150mm SSA BULK NO RECOVERY CORE SAMPLE TYPE GRAB MSHELBY TUBE SPLIT SPOON PENETRATION TESTS SAMPLE TYPE SYMBOL ELEVATION SAMPLE # DEPTH (m) SPT (Standard Pen Test) Ξ (Blows/300mm) 40 60 80 100 SOIL DESCRIPTION COMMENTS SPT (Total Unit Wt (kN/m³) SOIL 0 TOPSOIL G71 SILT - some clay, trace sand 232 - brown - soft, moist - low plasticity G72 CLAY - some silt, trace sand - mottled brown and dark brown G73 - firm, moist high plasticity G74 231 G75 firm below 1.5 m G76 G77 MCGILLIVRAYBLVD.GPJ UMA WINN.GDT soft below 2.1 m 230 END OF TEST HOLE AT 2.4 m IN CLAY 1. Seepage at 0.6 m from silt layer. 2. No sloughing. 3. Test hole backfilled with auger cuttings. THLOGS OF TEST HOL COMPLETION DEPTH: 2.44 m LOGGED BY: R. Belbas UMA | AECOM COMPLETION DATE: 5/5/08 REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: TH-08-A PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520494.677 E 629675.402 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 233.266 METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE BULK GRAB MSHELBY TUBE SPLIT SPOON SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ SYMBOL ELEVATION SAMPLE # ◆ SPT (Standard Pen Test) ◆
(Blows/300mm)
0 20 40 60 80 100 DEPTH (m) SOIL DESCRIPTION COMMENTS 0 40 60 8 SOIL (kN/m³) 18 19 18 20 F 60 -l₈₀ TOPSOIL 0 CLAY (Organic) - some silt, trace sand - black - stiff, moist G179 - high plasticity 233 G180 SILT - some clay, trace sand - brown - soft, moist - low plasticity G181 CLAY - some silt, trace sand, trace silt inclusions (< 20 mm dia.) - dark brown - stiff, moist high plasticity G182 232 G183 firm below 1.4 m G184 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G185 231 END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: TH-08-C PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520616.359 E 629897.83 ELEVATION (m): 233.148 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE BULK MSHELBY TUBE SPLIT SPOON SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL ELEVATION SAMPLE # ◆ SPT (Standard Pen Test) ◆ DEPTH (m) 2 (Blows/300mm) 0 40 60 80 100 Total Unit Wt SOIL DESCRIPTION COMMENTS (kN/m³) 18 19 TOPSOIL 0 SILT (Organic) - some clay, trace sand, trace gravel, black, soft, moist, low plasticity 233 G157 CLAY (Organic) - some silt, trace sand, trace gravel - black - firm, moist - intermediate to high plasticity G158 G159 CLAY - some silt, trace sand - dark brown - stiff, moist - high plasticity 25 to 50 mm dia. brown silt lense below 1.1 m G160 232 25 to 50 mm dia. brown silt lense below 1.4 m G161 G162 - firm below 1.8 m LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 G163 231 END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg TESTHOLE NO: TH-08-D LOCATION: N 5520713.048 E 630075.924 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 233.116 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA SHELBY TUBE NO RECOVERY CORE SPLIT SPOON BULK SAMPLE TYPE GRAB PENETRATION TESTS ★ Becker ★ SAMPLE TYPE . SYMBOL ♦ Dynamic Cone ♦ ELEVATION SAMPLE # DEPTH (m) ◆ SPT (Standard Pen Test) ◆ E (Blows/300mm)
10 40 60 80 100

Total Unit Wt (kN/m³)
7 18 19 20 21 SOIL DESCRIPTION COMMENTS SOIL TOPSOIL 0 G136 233 SILT - some clay, trace sand - brown - soft, moist - low plasticity G137 G138 CLAY - some silt, trace sand - mottled brown and dark brown - stiff, moist - high plasticity 232 G139 G140 - firm below 1.4 m G141 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 SILT G142 - brown - soft, moist, low plasticity 231 CLAY - some silt, trace sand - mottled brown and dark brown - stiff, moist, high plasticity END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-E CLIENT: City of Winnpeg LOCATION: N 5520810.824 E 630252.603 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 232.494 METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE SPLIT SPOON BULK MSHELBY TUBE SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ◇ Dynamic Cone ◇
 ◆ SPT (Standard Pen Test) ◆ SOIL SYMBOL ELEVATION SAMPLE # DEPTH (m) Z (Blows/300mm) 0 40 60 80 100 Total Unit Wt SOIL DESCRIPTION **COMMENTS** (kN/m³) 18 19 Liquid 80 100 MC 20 F 40 60 TOPSOIL 0 G120 SILT - some clay, trace sand - soft, moist - low plasticity CLAY - some silt, trace sand 232 G121 - dark brown stiff, moist - high plasticity G122 - firm below 0.8 m G123 G124 231 G125 5/22/08 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT G126 SILT - some clay, trace sand - brown (oxidized) - soft, wet, low plasticity CLAY - some silt, trace sand - dark brown - firm, moist - high plasticity 230 G127 LOGGED BY: R. Belbas COMPLETION DEPTH: 3.20 m UMA AECOM REVIEWED BY: N. Ferreira COMPLETION DATE: 5/5/08 PROJECT ENGINEER: Nelson Ferreira Page 1 of 2

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-E CLIENT: City of Winnpeg LOCATION: N 5520810.824 E 630252.603 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 232.494 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA CORE NO RECOVERY BULK GRAB MSHELBY TUBE SPLIT SPOON SAMPLE TYPE PENETRATION TESTS **※** Becker **※** SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL ELEVATION SAMPLE # ◆ Dynamic Cone ◆

◆ SPT (Standard Pen Test) ◆
(Blows/300mm)
0 20 40 60 80 100

■ Total Unit Wt ■
(kN/m³)
16 17 18 19 20 21

Plastic MC Liquid DEPTH (m) SOIL DESCRIPTION COMMENTS SPT END OF TEST HOLE AT 3.2 m IN CLAY 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 229 228 OG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 227 LOGGED BY: R. Belbas COMPLETION DEPTH: 3.20 m COMPLETION DATE: 5/5/08 UMA AECOM REVIEWED BY: N. Ferreira Page 2 of 2 PROJECT ENGINEER: Nelson Ferreira

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-F CLIENT: City of Winnpeg LOCATION: N 5520822.622 E 60314.938 PROJECT NO.: 0265-40600-210 ELEVATION (m): 233.343 CONTRACTOR: Paddock Drilling Ltd. METHOD: CT-250, 150mm SSA NO RECOVERY CORE SPLIT SPOON BULK MSHELBY TUBE SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL ELEVATION SAMPLE # ◆ SPT (Standard Pen Test) ◆ DEPTH (m) Ê (Blows/300mm) 40 60 80 100 COMMENTS SOIL DESCRIPTION 0 40 60 8

Total Unit Wt (kN/m³) 18 19 SAND AND GRAVEL (Fill) 0 - brown - loose to compact, moist - fine grained sand to coarse grained gravel - poorly graded G15 233 CLAY (Fill) - some silt, trace sand, trace gravel, trace organics G16 - dark brown and black - firm to stiff, moist to 0.6 m, frozen below 0.6 m - high plasticity G17 G18 232 G19 SILT - some clay, trace sand - brown - soft, moist - low plasticity G20 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 CLAY - some silt, trace sand, trace silt inclusions (< 10 mm dia.) - mottled brown and dark brown G21 - stiff, moist, - high plasticity END OF TEST HOLE AT 2.3 m IN CLAY 231 Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas REVIEWED BY: N. Ferreira COMPLETION DATE: 4/28/08 UMA **AECOM** PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: TH-08-H CLIENT: City of Winnpeg LOCATION: N 5520908.218 E 630426.875 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 232.603 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA NO RECOVERY CORE SPLIT SPOON BULK MSHELBY TUBE SAMPLE TYPE PENETRATION TESTS ★ Becker ★ SAMPLE TYPE ♦ Dynamic Cone ♦ SOIL SYMBOL **ELEVATION** SAMPLE # DEPTH (m) SPT (Standard Pen Test) Ξ (Blows/300mm)
0 40 60 80 100

Total Unit Wt SOIL DESCRIPTION COMMENTS (kN/m³) 18 19 Liquid 80 MC TOPSOIL G99 SILT - some clay, trace sand - brown - soft, moist - low plasticity G100 232 CLAY - some silt, trace sand - mottled brown and dark brown G101 - stiff, moist high plasticity G102 G103 firm below 1.4 m 231 G104 5/22/08 -2 SILT - some clay, trace sand LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT - brown (oxidized) G105 - soft, moist, low plasticity CLAY - some silt, trace sand - mottled brown and dark brown - stiff, moist, high plasticity END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. 230 COMPLETION DEPTH: 2.29 m LOGGED BY: R. Belbas UMA AECOM COMPLETION DATE: 5/5/08 REVIEWED BY: N. Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: TH-08-I PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnpeg LOCATION: N 5521003.757 E 630600.226 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 233.024 CONTRACTOR: Paddock Drilling Ltd. METHOD: YANMAR C25R, 150mm SSA CORE NO RECOVERY GRAB MSHELBY TUBE SPLIT SPOON BULK SAMPLE TYPE PENETRATION TESTS **※** Becker **※** SAMPLE TYPE ♦ Dynamic Cone ♦ SYMBOL ELEVATION SAMPLE # DEPTH (m) ◆ SPT (Standard Pen Test) ◆ (Standaro F C... (Blows/300mm) 40 60 80 100 \overline{z} SOIL DESCRIPTION COMMENTS SPT Total Unit Wt SOIL (kN/m³) 233 TOPSOIL 0 CLAY - some silt, trace sand, trace organics G85 - dark brown - stiff, moist - high plasticity SILT - some clay, trace sand - brown G86 - soft, moist - low plasticity G87 CLAY - some silt, trace sand, trace silt inclusions (< 5 mm dia.), trace sulfide inclusions (< 5 mm dia.) 232 - mottled brown and dark brown - stiff, moist - high plasticity G88 G89 - firm below 1.4 m G90 LOG OF TEST HOLE THLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 231 G91 END OF TEST HOLE AT 2.3 m IN CLAY Notes: 1. No seepage or sloughing. 2. Test hole backfilled with auger cuttings. LOGGED BY: R. Belbas COMPLETION DEPTH: 2.29 m REVIEWED BY: N. Ferreira COMPLETION DATE: 5/5/08 UMA AECOM PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg TESTHOLE NO: PH-08-01 LOCATION: N 5519952.636 E 628742.122 PROJECT NO.: 0265-40600-2100 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 234.118 METHOD: Diamond Core Drill and Hammer Drill NO RECOVERY CORE SHELBY TUBE SPLIT SPOON BULK SAMPLE TYPE GRAB ELEVATION SOIL SYMBOL DEPTH (m) SOIL DESCRIPTION COMMENTS 0 ASPHALT (75 mm) CONCRETE (200 mm) 234 SAND AND GRAVEL (Base) - dark brown - dense, dry to moist - fine grained sand to coarse grained gravel - poorly graded, sub angular CLAY (Fill) END OF PAVEMENT HOLE AT 0.6 m IN CLAY FILL 1. Pavement hole cored with a diamond core drill equipped with a 110 mm core bit. Road base drilled with a 0.6 m long Hilti hammer drill equipped with a 50 mm drill bit. Pavement hole cored and drilled 0.2 m from edge of pavement. LOG OF TEST HOLE PHLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 3. Core samples of asphalt and concrete obtained and transported to UMA's material testing laboratory.

4. Pavement hole backfilled with cold patch. COMPLETION DEPTH: 0.36 m LOGGED BY: R. Belbas COMPLETION DATE: 4/27/08 UMA | AECOM REVIEWED BY: Nelson Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

TESTHOLE NO: PH-08-03 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520121.741 E 628999.844 ELEVATION (m): 233.873 CONTRACTOR: Paddock Drilling Ltd. METHOD: Diamond Core Drill and Hammer Drill CORE BULK NO RECOVERY SPLIT SPOON GRAB MSHELBY TUBE SAMPLE TYPE SOIL SYMBOL ELEVATION DEPTH (m) SOIL DESCRIPTION COMMENTS ASPHALT (75 mm) 0 CRUSHED LIMESTONE (Base) - some silt - light brown - dense, moist - fine grainded sand to coarse grained gravel - poorly graded, sub angular Minimum road base END OF PAVEMENT HOLE AT 0.6 m IN CRUSHED LIMESTONE BASE depth 0.6 m Notes: OG OF TEST HOLE PHLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 1. Pavement hole cored with a diamond core drill equipped with a 110 mm core bit. Road base drilled with a 0.6 m long Hilti hammer drill equipped with a 50 mm drill bit. Pavement hole cored and drilled 1.0 m from edge of pavement. 3. Core samples of asphalt and concrete obtained and transported to UMA's material testing laboratory. 4. Pavement hole backfilled with cold patch. 233 COMPLETION DEPTH: 0.61 m LOGGED BY: R. Belbas COMPLETION DATE: 4/27/08 UMA AECOM REVIEWED BY: Nelson Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: PH-08-05 CLIENT: City of Winnipeg LOCATION: N 5520387.296 E 629366.765 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 233.455 CONTRACTOR: Paddock Drilling Ltd. METHOD: Diamond Core Drill and Hammer Drill CORE NO RECOVERY SPLIT SPOON BULK SHELBY TUBE SAMPLE TYPE GRAB SOIL SYMBOL ELEVATION DEPTH (m) SOIL DESCRIPTION COMMENTS ASPHALT (115 mm) 0 CRUSHED LIMESTONE (Base) - some silt - light brown - dense, moist fine grainded sand to coarse grained gravel
 poorly graded, sub angular 233 Minimum road base END OF PAVEMENT HOLE AT 0.6 m IN CRUSHED LIMESTONE BASE depth 0.6 m LOG OF TEST HOLE PHLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 1. Pavement hole cored with a diamond core drill equipped with a 110 mm core bit. Road base drilled with a 0.6 m long Hilti hammer drill equipped with a 50 mm drill bit. Asphalt and road base cored and drilled 0.4 m from edge of pavement. 3. Core samples of asphalt and concrete obtained and transported to UMA's material testing laboratory. 4. Pavement hole backfilled with cold patch. COMPLETION DEPTH: 0.61 m LOGGED BY: R. Belbas REVIEWED BY: Nelson Ferreira COMPLETION DATE: 4/27/08 UMA AECOM PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: PH-08-10 CLIENT: City of Winnipeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520536.242 E 629803.510 CONTRACTOR: Paddock Drilling Ltd. ELEVATION (m): 233.876 METHOD: Diamond Core Drill and Hammer Drill NO RECOVERY CORE SHELBY TUBE SPLIT SPOON BULK SAMPLE TYPE GRAB SOIL SYMBOL ELEVATION DEPTH (m) SOIL DESCRIPTION COMMENTS CONCRETE (190 mm) 0 ASPHALT (150 mm) SAND AND GRAVEL (Base) - dark brown - dense, dry to moist fine grained sand to coarse grained gravel
 poorly graded, sub angular Minimum road base END OF PAVEMENT HOLE AT 0.6 m IN SAND AND GRAVEL BASE depth 0.6 m Notes: 1. Pavement hole cored with a diamond core drill equipped with a 110 mm core bit. Road base drilled with a 0.6 m long Hilti hammer drill equipped with a 50 mm drill bit. 2. Concrete, asphalt and road base cored and drilled 0.2 m from edge of pavement. 3. Core samples of asphalt and concrete obtained and transported to UMA's material testing laboratory. 4. Pavement hole backfilled with cold patch. 233 -LOGGED BY: R. Belbas COMPLETION DEPTH: 0.61 m UMA AECOM COMPLETION DATE: 4/27/08 REVIEWED BY: Nelson Ferreira PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

OG OF TEST HOLE PHLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08

TESTHOLE NO: PH-08-15 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg PROJECT NO.: 0265-40600-2100 LOCATION: N 5520764.875 E 630228.886 ELEVATION (m): 233.592 CONTRACTOR: Paddock Drilling Ltd. METHOD: Diamond Core Drill and Hammer Drill NO RECOVERY CORE SPLIT SPOON BULK SHELBY TUBE SAMPLE TYPE GRAB SOIL SYMBOL ELEVATION DEPTH (m) SOIL DESCRIPTION COMMENTS CONCRETE (190 mm) ASPHALT (75 mm) SAND AND GRAVEL (Base) - dark brown - dense, dry to moist - fine grained sand to coarse grained gravel - poorly graded, sub angular 233 Minimum road base END OF PAVEMENT HOLE AT 0.6 m IN SAND AND GRAVEL BASE depth 0.6 m Notes: LOG OF TEST HOLE PHLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 1. Pavement hole cored with a diamond core drill equipped with a 110 mm core bit. Road base drilled with a 0.6 m long Hilti hammer drill equipped with a 50 mm drill bit. 2. Concrete, asphalt and road base cored and drilled 0.30 m from edge of pavement. 3. Core samples of asphalt and concrete obtained and transported to UMA's material testing laboratory.

4. Pavement hole backfilled with cold patch. COMPLETION DEPTH: 0.61 m LOGGED BY: R. Belbas COMPLETION DATE: 4/27/08 UMA AECOM REVIEWED BY: Nelson Ferreira Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

				CLIENT: City of Winnipeg TES				THOLE NO: PH-08-	17
LOCATION: N					JECT NO.: 0265-40	600-21			
	: Paddock Drilling Ltd.				Core Drill and			VATION (m): 233.64	14
SAMPLE TYPE	GRAB	SHELBY TUBE	SPLIT SPO	OON E	BULK	✓NO	RECOVE	RY CORE	
DEPTH (m) SOIL SYMBOL	SOIL DESC			CRIPTION				COMMENTS	ELEVATION
0 .4.4 CO	NCRETE (175 mm)					sa)			
ASF	PHALT (50 mm)							× ,	. 3
- da - de - fin	ID AND GRAVEL (Base) rk brown nse, dry to moist e grained sand to coarse graine orly graded, sub angular	d gravel							
CLA	Y (Fill)								
Note 1. Po equi 2. Co 3. Co	avement hole cored with a diam pped with a 50 mm drill bit. pocrete, asphalt and road base	ond core drill equipped with a 11 cored and drilled 0.2 m from edg crete obtained and transported to	e of pavement.			long Hilti hamme	r drill		233
1				LOGGED F	BY: R. Belbas		COMPLI	ETION DEPTH: 0.36 m	
	UMA AECOM				BY: Nelson F			ETION DATE: 4/27/08	
					ENGINEER: N				e 1 of

PROJECT: McGillivray Boulevard Widening TESTHOLE NO: PH-08-19 CLIENT: City of Winnipeg LOCATION: N 5520959.239 E 630575.690 PROJECT NO.: 0265-40600-2100 ELEVATION (m): 233.933 CONTRACTOR: Paddock Drilling Ltd. METHOD: Diamond Core Drill and Hammer Drill CORE NO RECOVERY SPLIT SPOON BULK GRAB MSHELBY TUBE SAMPLE TYPE SOIL SYMBOL ELEVATION DEPTH (m) SOIL DESCRIPTION COMMENTS CONCRETE (215 mm) ASPHALT (90 mm) SAND AND GRAVEL (Base) - dark brown - dense, dry to moist - fine grained sand to coarse grained gravel - poorly graded, sub angular Minimum road base END OF PAVEMENT HOLE AT 0.6 m IN SAND AND GRAVEL BASE depth 0.6 m 1. Pavement hole cored with a diamond core drill equipped with a 110 mm core bit. Road base drilled with a 0.6 m long Hilti hammer drill equipped with a 50 mm drill bit. 2. Concrete, asphalt and road base cored and drilled 0.3 m from edge of pavement. 3. Core samples of asphalt and concrete obtained and transported to UMA's material testing laboratory. 4. Pavement hole backfilled with cold patch. 233 -LOGGED BY: R. Belbas COMPLETION DEPTH: 0.61 m UMA AECOM REVIEWED BY: Nelson Ferreira COMPLETION DATE: 4/27/08 PROJECT ENGINEER: Nelson Ferreira Page 1 of 1

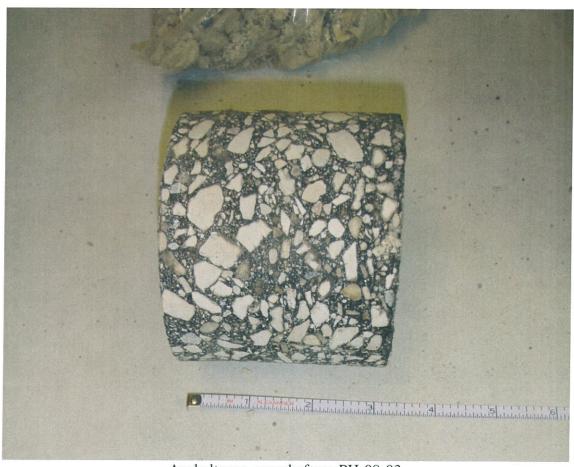
LOG OF TEST HOLE PHLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08

TESTHOLE NO: PH-08-21 PROJECT: McGillivray Boulevard Widening CLIENT: City of Winnipeg LOCATION: N 5521071.265 E 630689.325 PROJECT NO.: 0265-40600-2100 METHOD: Diamond Core Drill and Hammer Drill ELEVATION (m): 233.688 CONTRACTOR: Paddock Drilling Ltd. NO RECOVERY CORE BULK SHELBY TUBE SPLIT SPOON SAMPLE TYPE GRAB SOIL SYMBOL **ELEVATION** DEPTH (m) SOIL DESCRIPTION COMMENTS CONCRETE (150 mm) 0 ASPHALT (75 mm) SAND AND GRAVEL (Base) - dark brown - dense, dry to moist - fine grained sand to coarse grained gravel - poorly graded, sub angular CLAY (Fill) END OF PAVEMENT HOLE AT 0.4 m IN CLAY FILL Notes: LOG OF TEST HOLE PHLOGS_MCGILLIVRAYBLVD.GPJ UMA WINN.GDT 5/22/08 1. Pavement hole cored with a diamond core drill equipped with a 110 mm core bit. Road base drilled with a 0.6 m long Hilti hammer drill equipped with a 50 mm drill bit. Concrete, asphalt and road base cored and drilled 0.3 m from edge of pavement. 233 -3. Core samples of asphalt and concrete obtained and transported to UMA's material testing laboratory. 4. Pavement hole backfilled with cold patch. COMPLETION DEPTH: 0.38 m LOGGED BY: R. Belbas COMPLETION DATE: 4/27/08 UMA AECOM REVIEWED BY: Nelson Ferreira Page 1 of 1 PROJECT ENGINEER: Nelson Ferreira

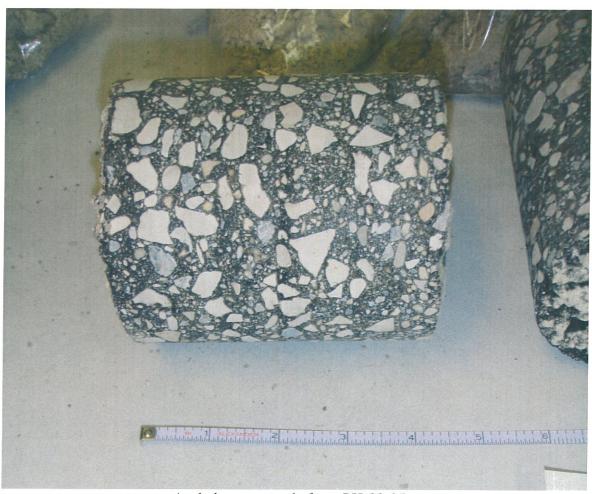
Appendix B
Photos of Core Samples



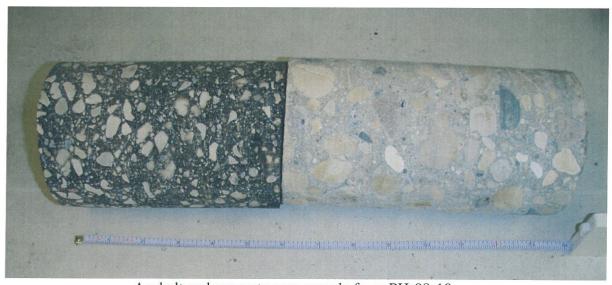
Asphalt and concrete core sample from PH-08-01



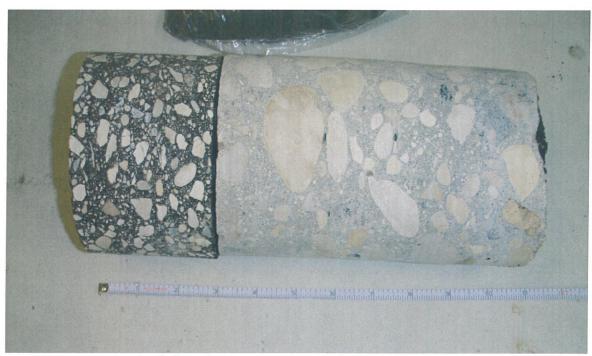
Asphalt core sample from PH-08-03



Asphalt core sample from PH-08-05



Asphalt and concrete core sample from PH-08-10



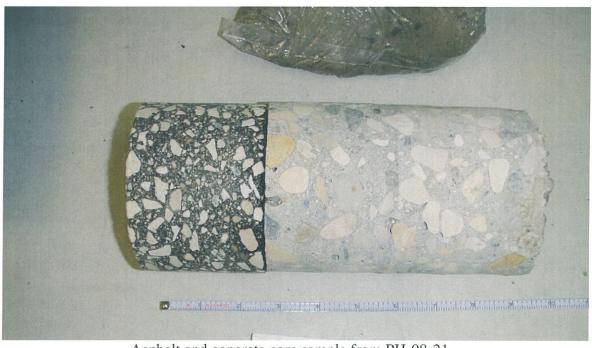
Asphalt and concrete core sample from PH-08-15



Asphalt and concrete core sample from PH-08-17



Asphalt and concrete core sample from PH-08-19



Asphalt and concrete core sample from PH-08-21